LI RESIDENCE CUSTOM

WEAVER CONSTRUCTION

4657 86TH AVE. SE MERCER ISLAND, WA 98040 **PARCEL NUMBER #759810-0545**

ARBORIST:

PERSPECTIVE IMAGE



PROJECT PARTICIPANTS

OWNER:	PAUL LI PH: (703) 965-9722	JURISDICTION:
CONTRACTOR:	WEAVER CONSTRUCTION WILLIAM WEAVER PH: (408) 348-3095	PROJECT ADRESS:
ENGINEERING:	CSES	TAX ACCOUNT NO.: · · · · · · #759810-0545
	EVAN APOLIS 6311 17TH. AVE NE SEATTLE, WA 98115 PH: 206-527-1288	PROPERTY TYPE: · · · · · · · · · · RESIDENTIAL (R) ZONING.: · · · · · · · · · · · · · R 9.6
DESIGNER / DRAFTSMAN:	LURE DESIGN SOLUTIONS	LOT SIZE: 10,000 SF23 ACRES
	SHAWN SULLIVAN 4402 242ND PLACE SW	OCCUPANCY GROUP: · · · · · · · R
	MOUNTLAKE TERRACE, WA	CONSTRUCTION TYPE TYPE V-NR
	98043 PH: (425) 870-0383	SEWER: IN STREET
CIVIL ENGINEER:	CANON	WATER: IN STREET
OIVIL LINGINLLIN.		OAO IN OTDEET

11415 NE 128TH ST. SUITE 110 KIRKLAND, WA 98034 PH: (425)-820-3420: **SQUARE FOOTAGE**

KATIE ROLLINS PH: (310) 382-5133

BENJAMIN MARK

(INCLUDES 82 SF / 500 SF ADU)	2364 SF
UPPER FLOOR	1623 SF
TOTAL	3987 SF
GARAGE	 580 SF
REAR COVERED PATIO	334 SF

LEGAL DESCRIPTION

PROJECT DESCRIPTION

PROJECT DATA

MERCER ISLAND , WA 98040

TIGHTLINE/SANITARY SEWER

BUILDING /I

ATTACHED GARAGE, THE EXISTING FOUNDATION AND LOWER FLOOR CONCRETE SLAB WILL BE REMAIN AND BE MODIFIED AS REQUIRED TO ACCOMMODATE THE NEW STRUCTURE. TREES ARE A PROPOSED NEW 3987 SQUARE TWO STORY RESIDENCE W/

· · · · · · · · · · · · DOWNSPOUT TO

AN ATTACHED (2) CAR GARAGE, REAR COVERED PATIO AND A 500 SF ADU WILL BE CONSTRUCTED. SITE IMPROVEMENTS TO INCLUDE A NEW CONCRETE DRIVEWAY AND A NEW SIDEWALK ACCESSING THE MAIN ENTRY TO THE HOME.

CODE SUMMARY

- MERCER ISLAND CITY CODE (MICC) SCHMIDS VITUS E SEATTLE ACRE TRS S 80 FT OF E 125 FT - 2018 INTERNATIONAL BUILDING CODE (IRC) - 2018 INTERNATIONAL MECHANICAL CODE (IMC) - 2018 INTERNATIONAL FIRE CODE (IMC) - WASHINGTON STATE ENERGY CODE (WCEC) - 2018 UNIFORM PLUMBING CODE CODE (UPC)

ARCHITECTURAL

DRAWING INDEX

ARCHITE	CTURAL
40	COVER
A0	
A1.0	EXCLUDED GFA -DIAGRAM AND CALCS
A1.1	
, _	GFA -DIAGRAM AND CALCS
	LOT COVERAGE -DIAGRAM AND CALCS
	HARDSCAPE -DIAGRAM AND CALCS
	AVERAGE BUILDING HEIGHT STUDY
	LOWER FLOOR PLAN
	UPPER FLOOR PLAN
	ROOF PLAN
,	EXTERIOR ELEVATIONS
	EXTERIOR ELEVATIONS
A4.1	BUILDING SECTIONS
A4.2	BUILDING SECTIONS
_	BUILDING SECTIONS
A4.4	BUILDING SECTIONS
_	WALL SECTIONS
_	WALL SECTIONS
	WALL SECTIONS
A6.1	WINDOW AND DOOR SCHEDULE
A6-2	WINDOW AND DOOR SYSTEMS / TYPES
A7.1	STAIR PLANS AND ELEVATIONS
(A7.2 <	STAIR DETAILS
A7.3	STAIR DETAILS
(A7.4)	STAIR DETAILS

SHT-1 TOPO-SURVEY **STRUCTURAL**

SURVEY

S1	FOUNDATION PLAN
S2	UPPER FLOOR FRAMING AND GROUND FLOOR WALL PLAN
S3	ROOF FRAMING AND UPPER FLOOR WALL PLAN
S4	STRUCTURAL DETAILS
S5	STRUCTURAL DETAILS

STRUCTURAL DETAILS /SHEAR WALL SCHEDULE

CIVAL

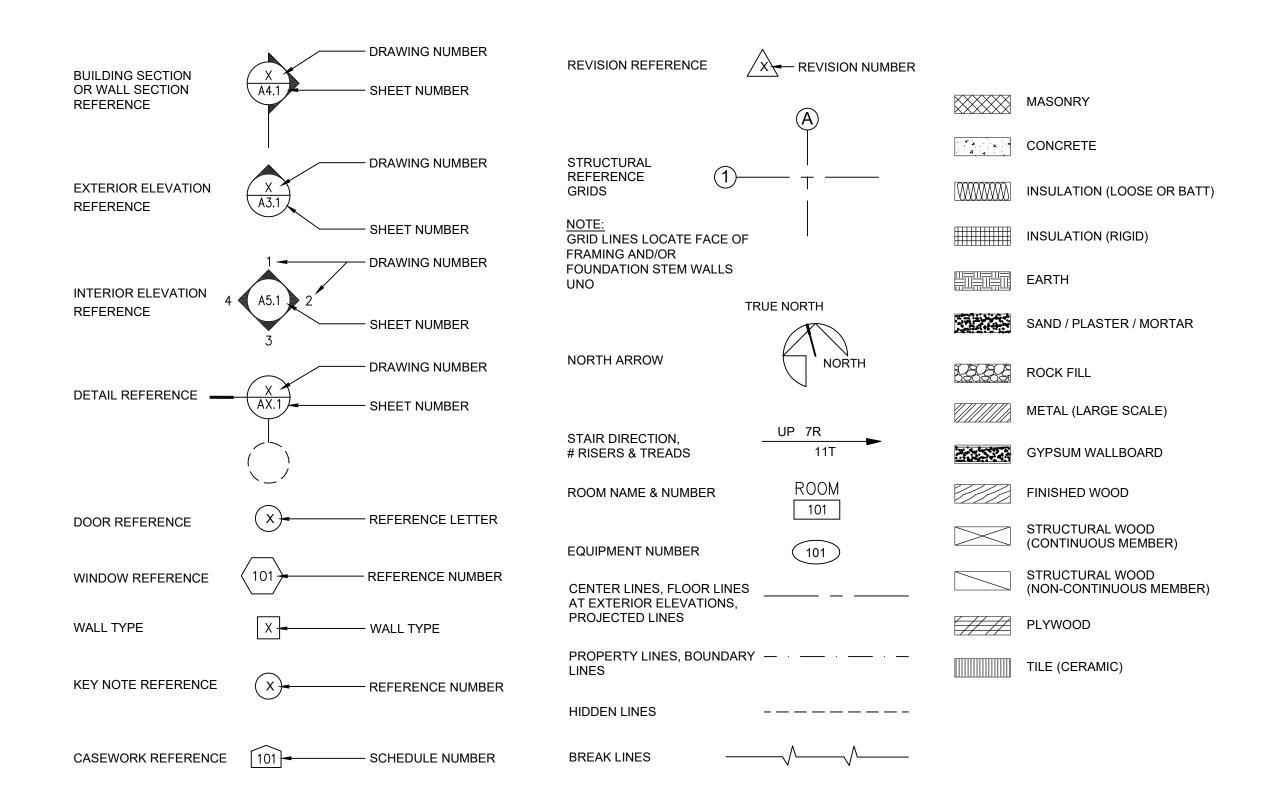
C1	TITLE SHEET
C2	DRAINAGE AND BMP PLAN
C3	UTILITY CONNECTIONS PLAN
C4	GRADING AND DRAINAGE DETAILS
C5	TESC PLAN
C6	TESC NOTES
C7	TESC DETAILS

TREE REPLANTING PLAN

C8 TESC DETAILS

L1.0 TREE REPLANTING PLAN

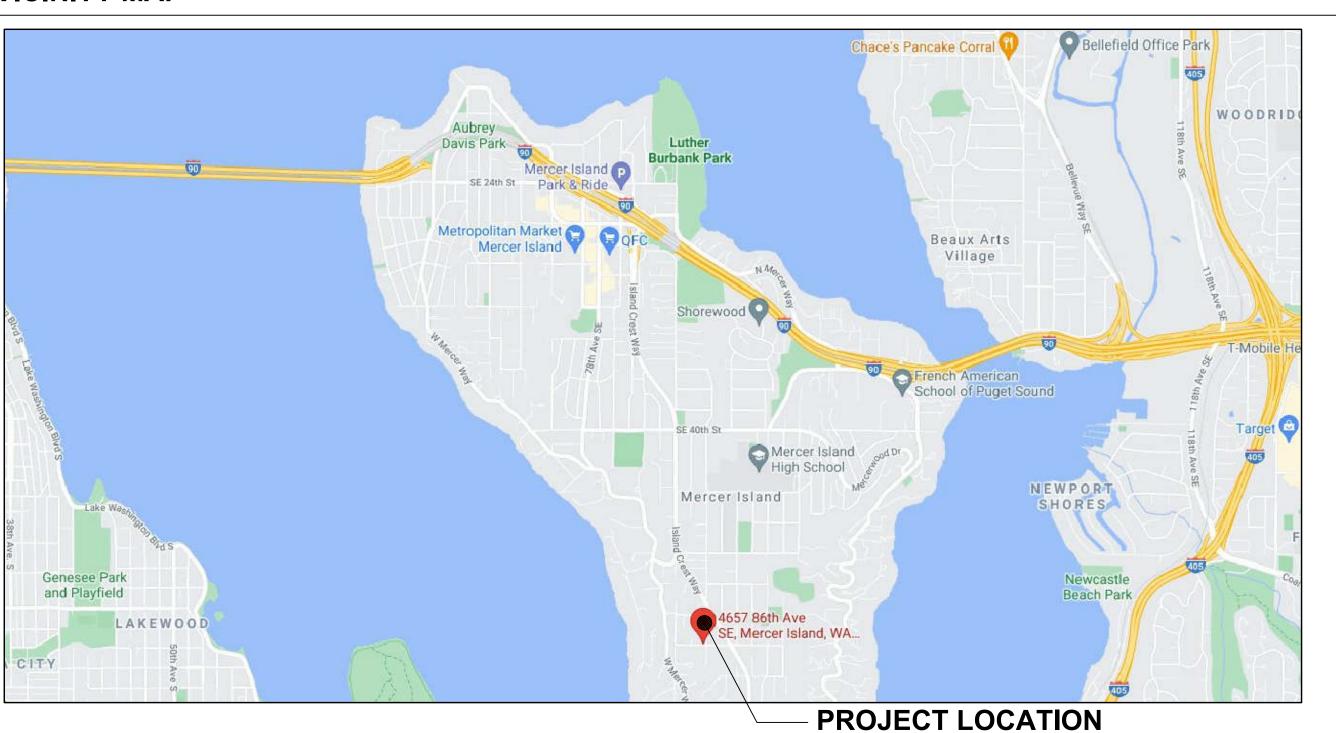
SYMBOLS



VICINITY MAP

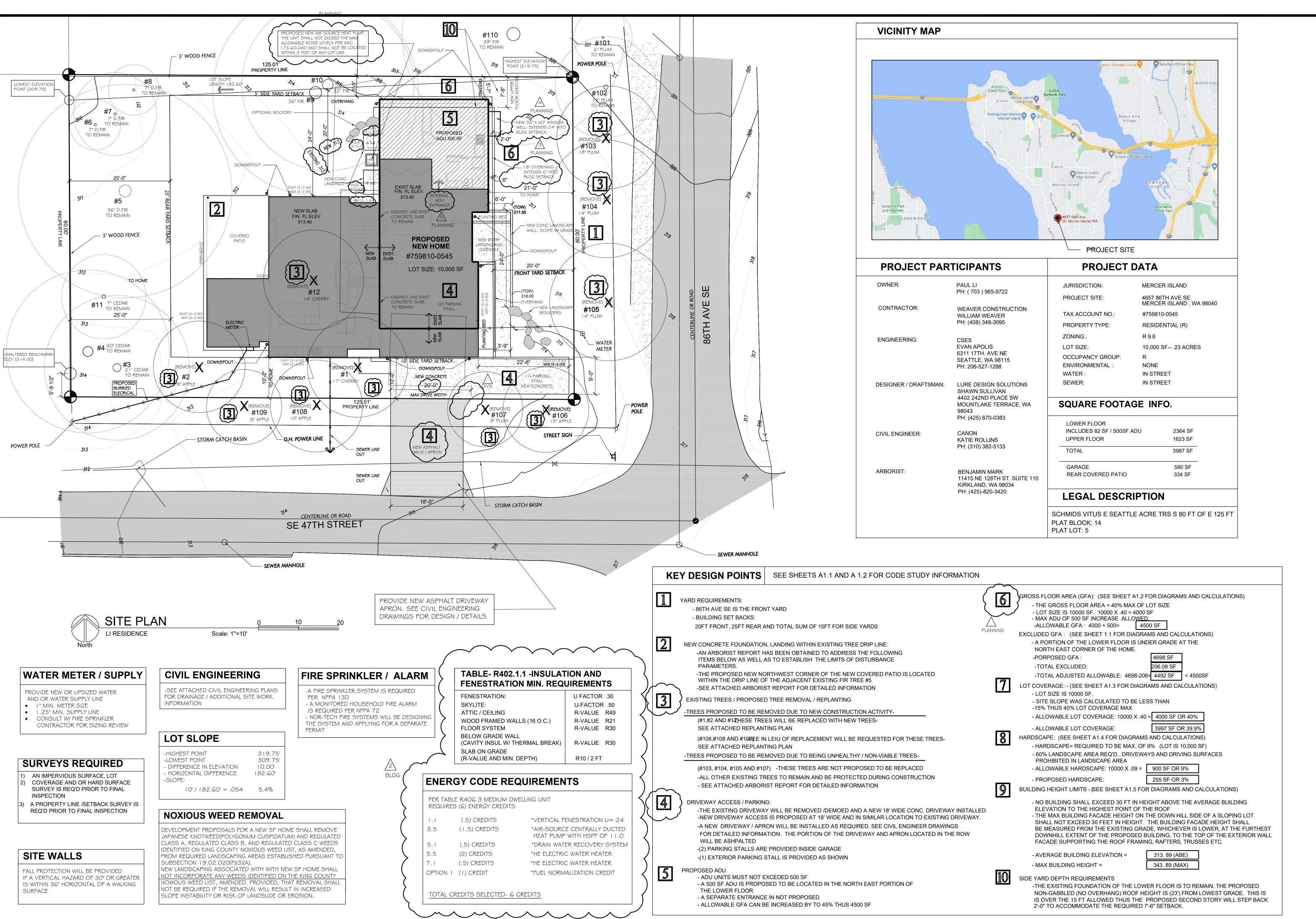
PLAT BLOCK: 14

PLAT LOT: 5



FINAL CD SET 10-14-2022 PERMIT REV 03-20-2023

DATE: 01-04- 2022 DESIGNED: DRAWN: 2022- 01 JOB NO: SHEET:



Misc. Info: FINAL CD SET 10-14-2022 PERMIT REV 03-20-2023

PERMIT

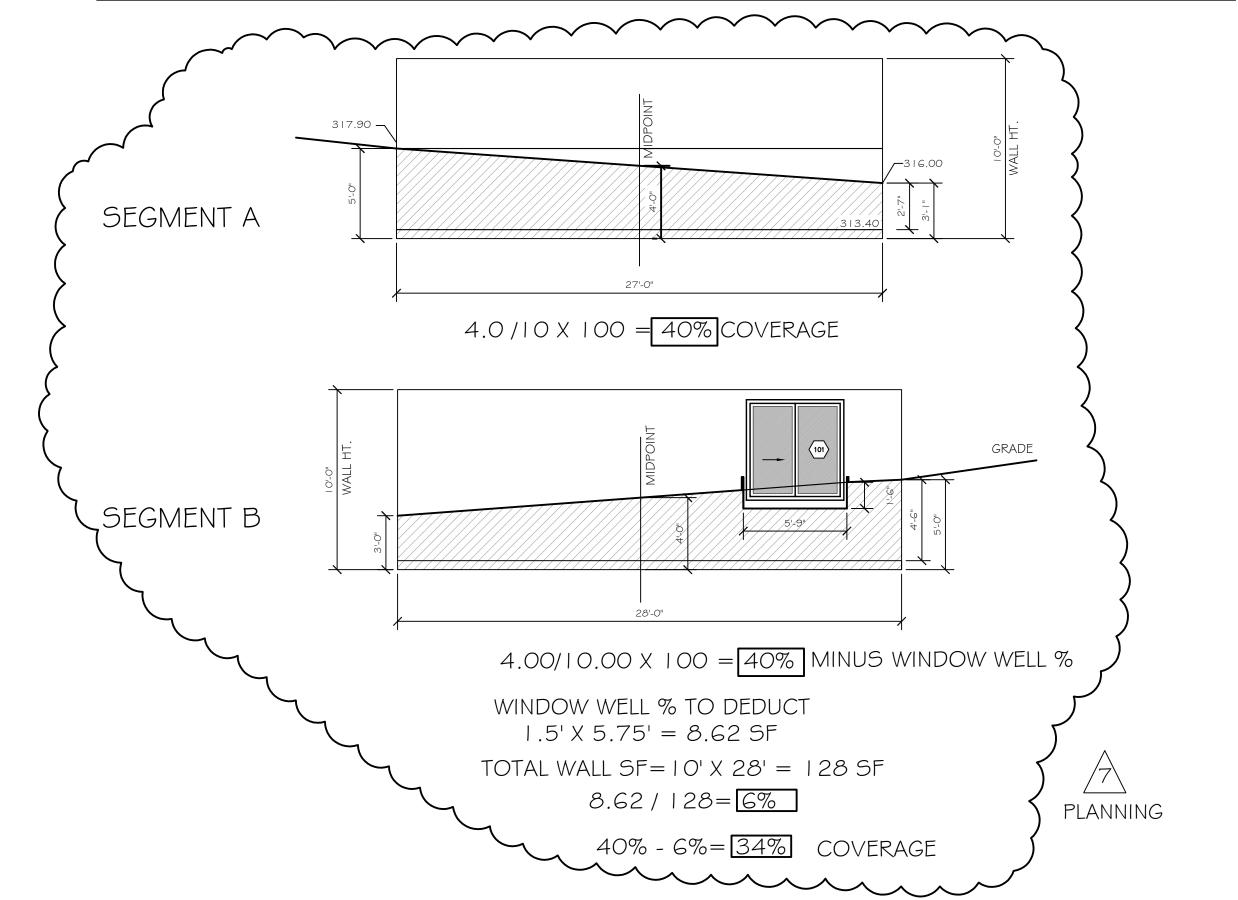
SE

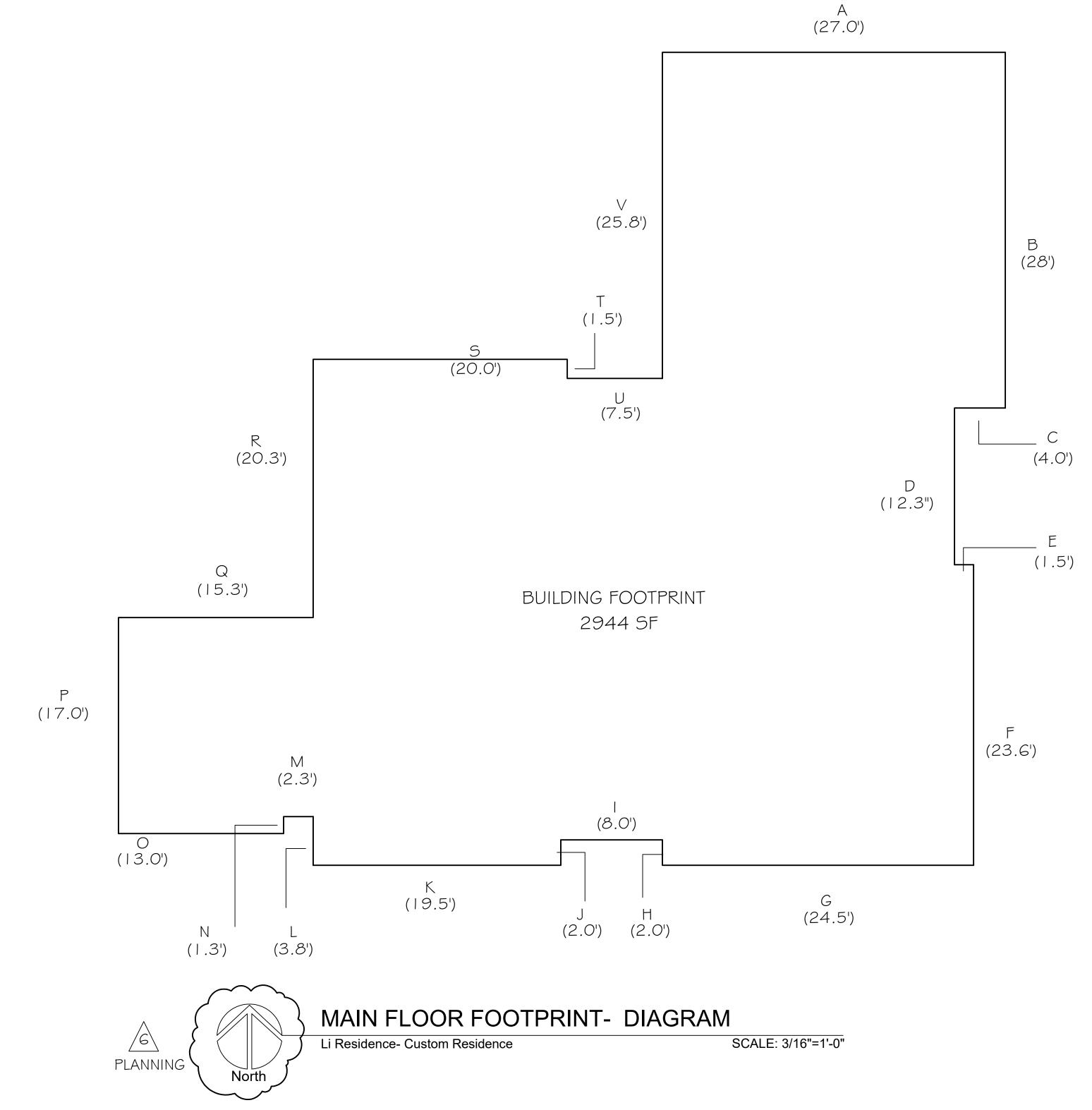
DATE: 01-04- 2022 DESIGNED: 2022- 01 JOB NO:

SHEET:

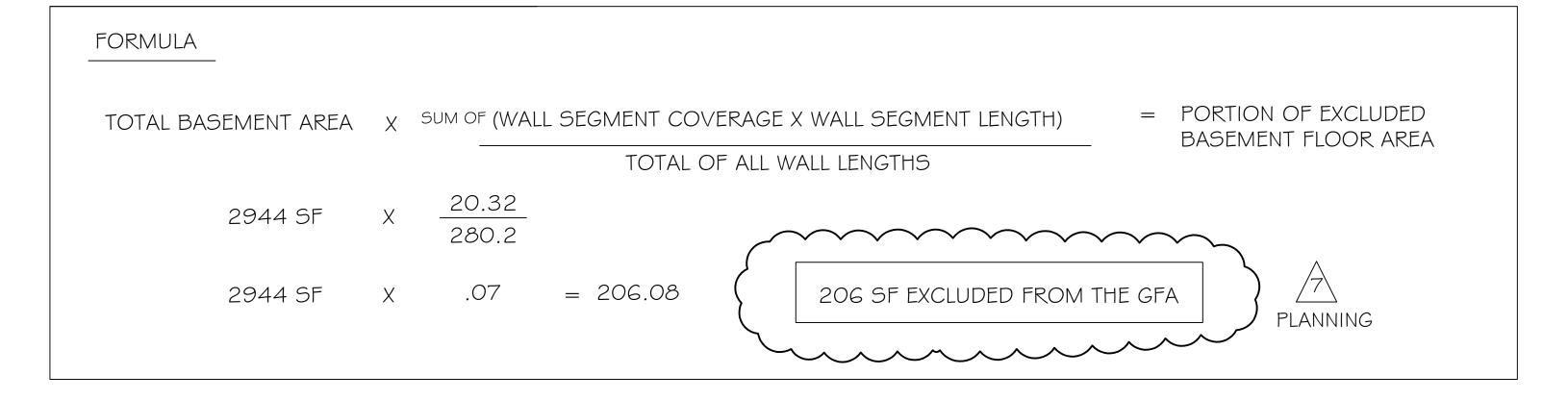
TABLE OF WALL LENGTHS AND COVERAGES

WALL SEGMENTS	WALL LENGTH X	COVERAGE	= RESULTS	
А	27.0'	40%	(10.80	
В	28.0'	(34%)	9.52	
С	4.0'	0%	0%	
D	12.3'	0%	0%	
E	1.5'	0%	0%	
F	23.6'	0%	0%	
G	24.5'	0%	0%	
Н	2.0'	0%	0%	
I	8.0'	0%	0%	
J	2.0'	0%	0%	
K	19.5'	0%	0%	
L	3.8'	0%	0%	
M	2.3'	0%	0%	
N	1.3'	0%	0%	
O	13.0'	0%	0%	
Р	17.0'	0%	0%	
Q	15.3'	0%	0%	
R	20.3'	0%	0%	
S	20.0'	0%	0%	
Т	1.5'	0%	0%	
U	7.5'	0%	0%	
V	25.8'	0%	0%	
TOTAL	280.20'	N/A'	(20.32% OF WALLS BELOW GR	RADE





EXLUDEDED GFA CACULATIONS



Misc. Info:
1. FINAL CD SET 10-14-2022
2. PERMIT REV 03-20-2023
3.
4.

3. 4. 5.

PERMIT SET

Shawn Sullivan Designer

402 242ND PLACE SW
MOUNTLAKE TERRACE, WA 98043

LI RESIDENCE
CUSTOM RESIDENCE
4657 86TH AVE. SE
MERCER ISLAND, WA 98040

CLUDED GFA LCULATIONS

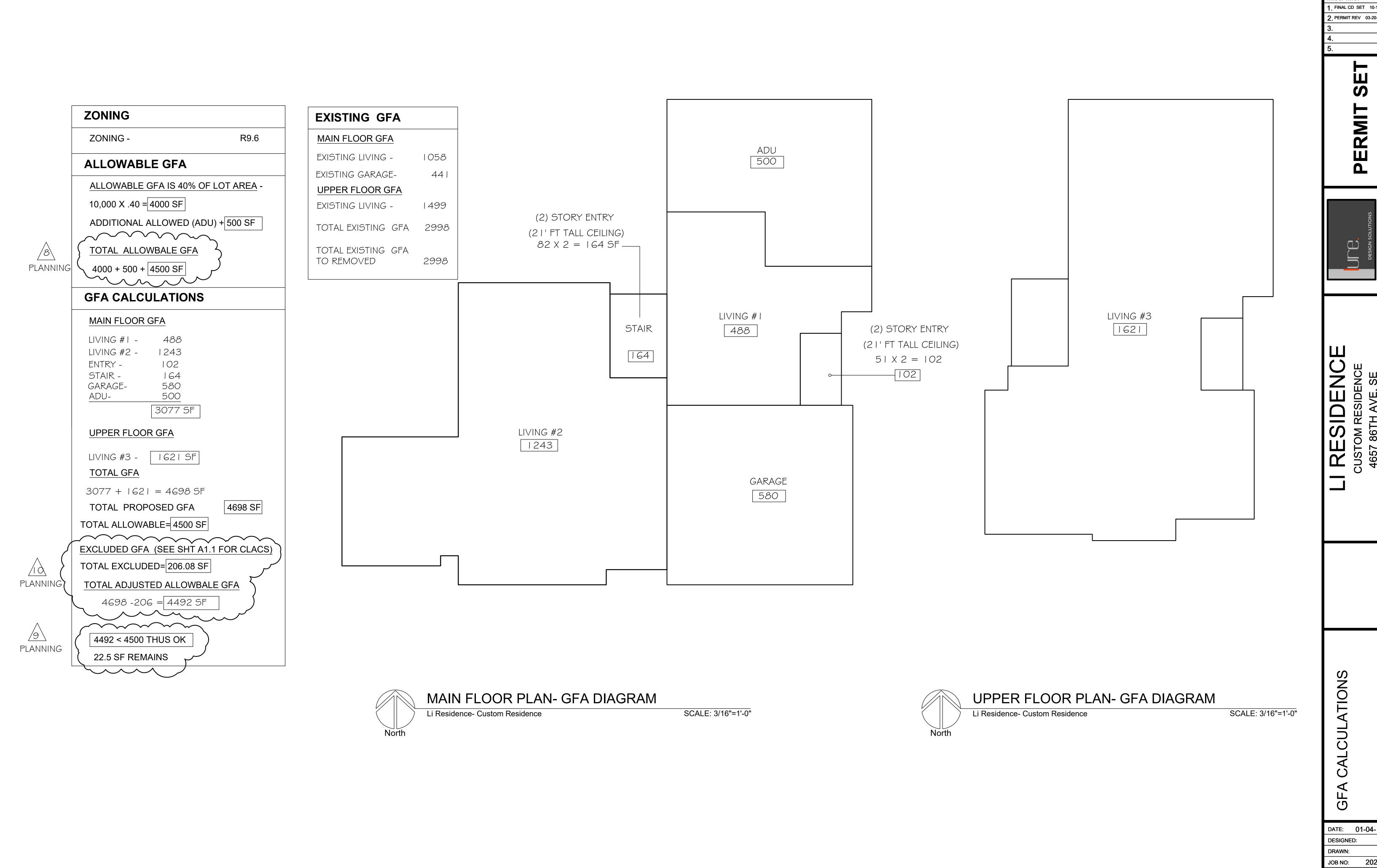
DATE: 01-04- 2022

DESIGNED: SLS

DRAWN: SLS

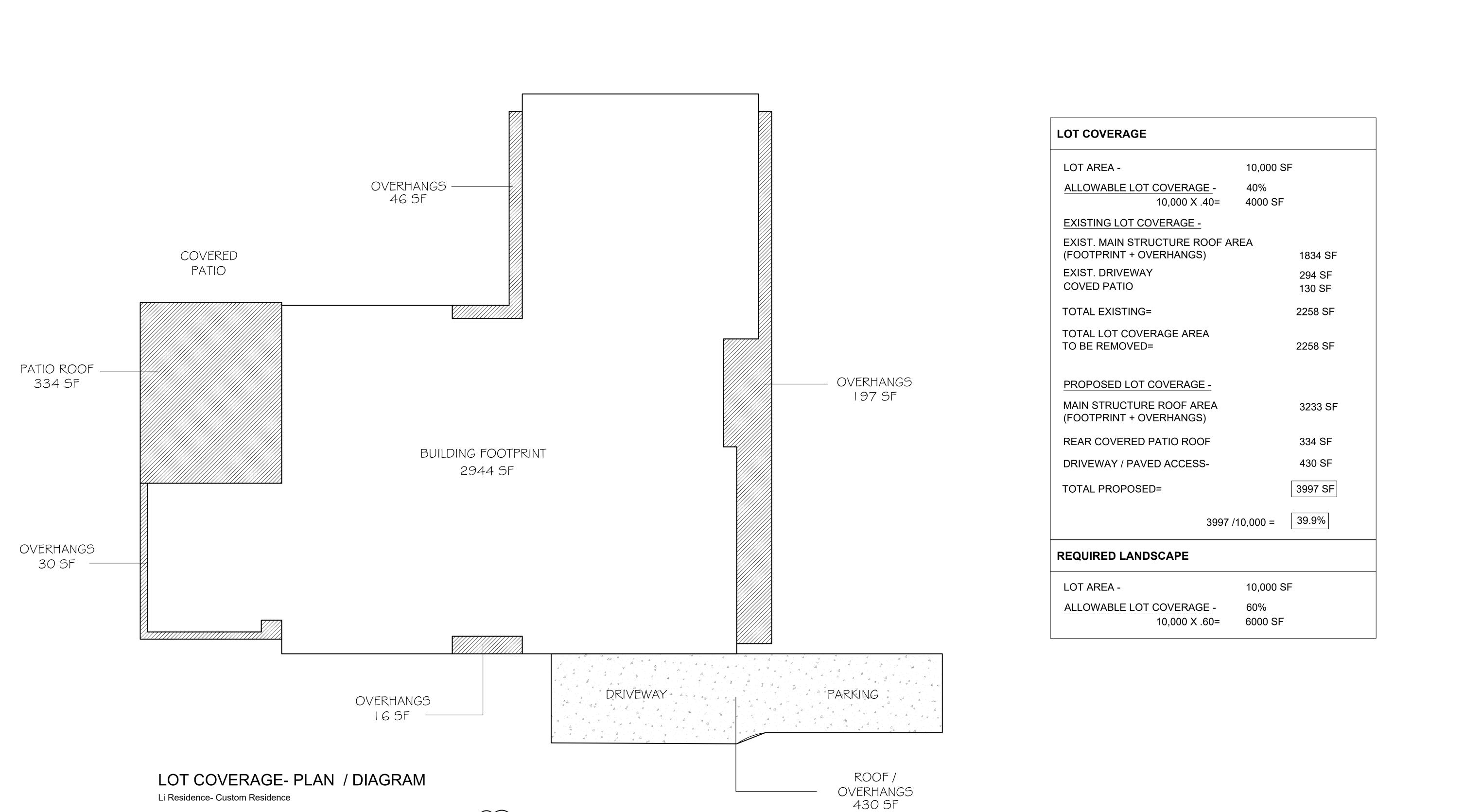
JOB NO: 2022- 01

SHEET:



1 FINAL CD SET 10-14-2022 PERMIT REV 03-20-2023

DATE: 01-04- 2022 JOB NO: 2022- 01



SCALE: 3/16"=1'-0"

LOT COVERAGE- DIAGRAM

Li Residence- Custom Residence

HATCH REPRESENTS

DRIVEWAY / PARKING

HATCH REPRESENTS

(ROOF OVERHANGS AND BUILDING CANTILEVERS ETC.)

Misc. Info:
1. FINAL CD SET 10-14-2022
2. PERMIT REV 03-20-2023
3.
4.

PERMIT SET

Shawn Sullivan Designer
MOUNTLAKE TERRACE, WA 98043

CUSTOM RESIDENCE
4657 86TH AVE. SE

OT COVERAGE

JIAGRAM / CALCS

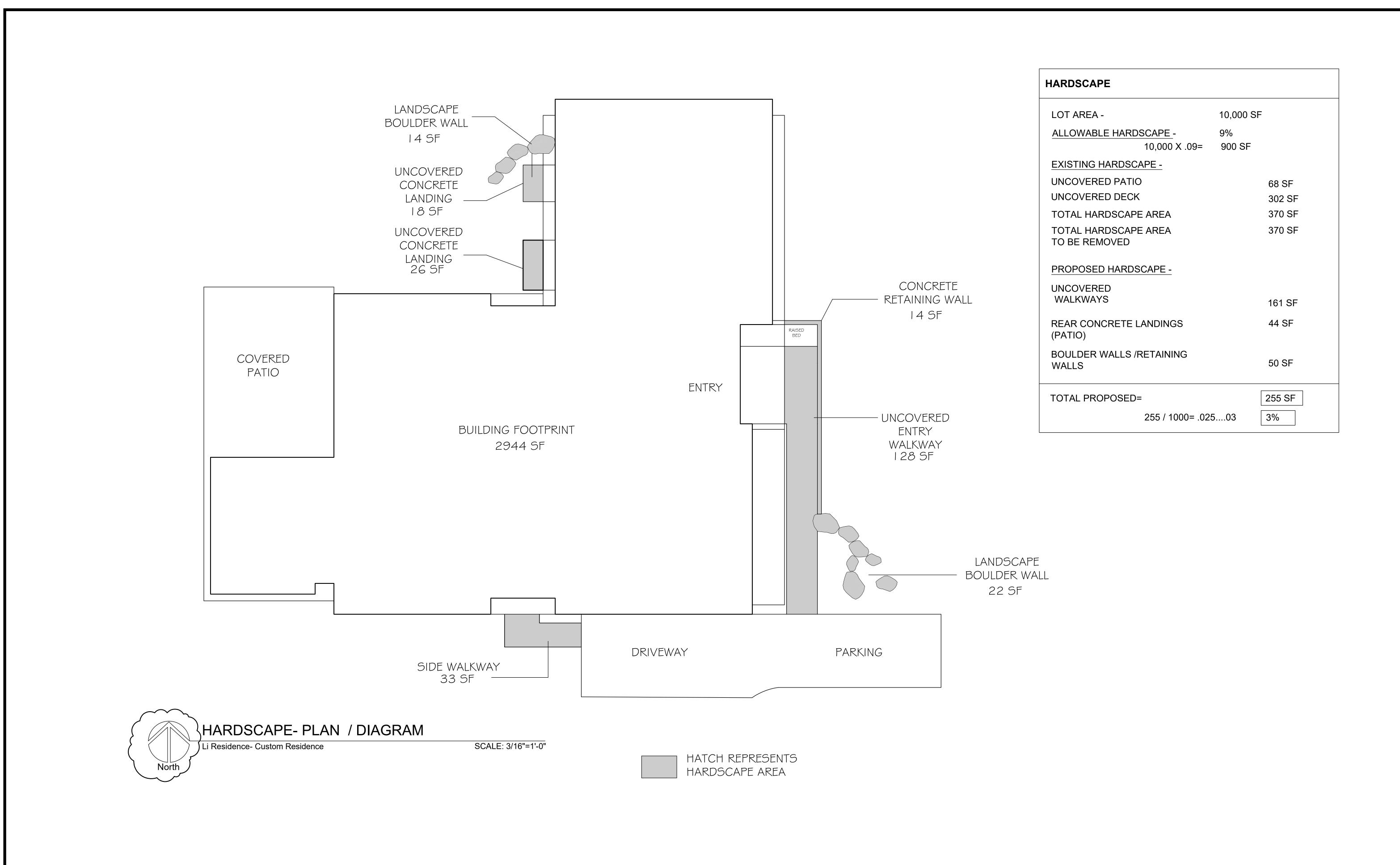
DATE: 01-04- 2022

DESIGNED: SLS

DRAWN: SLS

JOB NO: 2022- 01

SHEET:



Misc. Info:
1. FINAL CD SET 10-14-2022
2. PERMIT REV 03-20-2023
3.
4.

4. 5.

PERMIT SET

DESIGN SOLUTIONS

Shawn Sullivan Designer

4402 242ND PLACE SW
MOUNTLAKE TERRACE, WA 98043
425-870-0383

SIDENCE M RESIDENCE 6TH AVE. SE

CUSTOM RESI 4657 86TH AV MERCER ISLAND,

RDSCAPE COVERAGE

DATE: 01-04- 2022

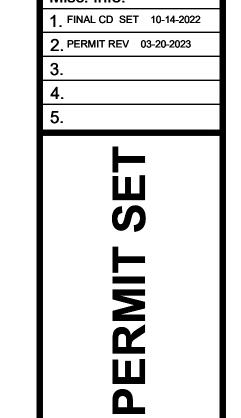
DESIGNED: SLS

DRAWN: SLS

JOB NO: 2022- 01

SHEET:

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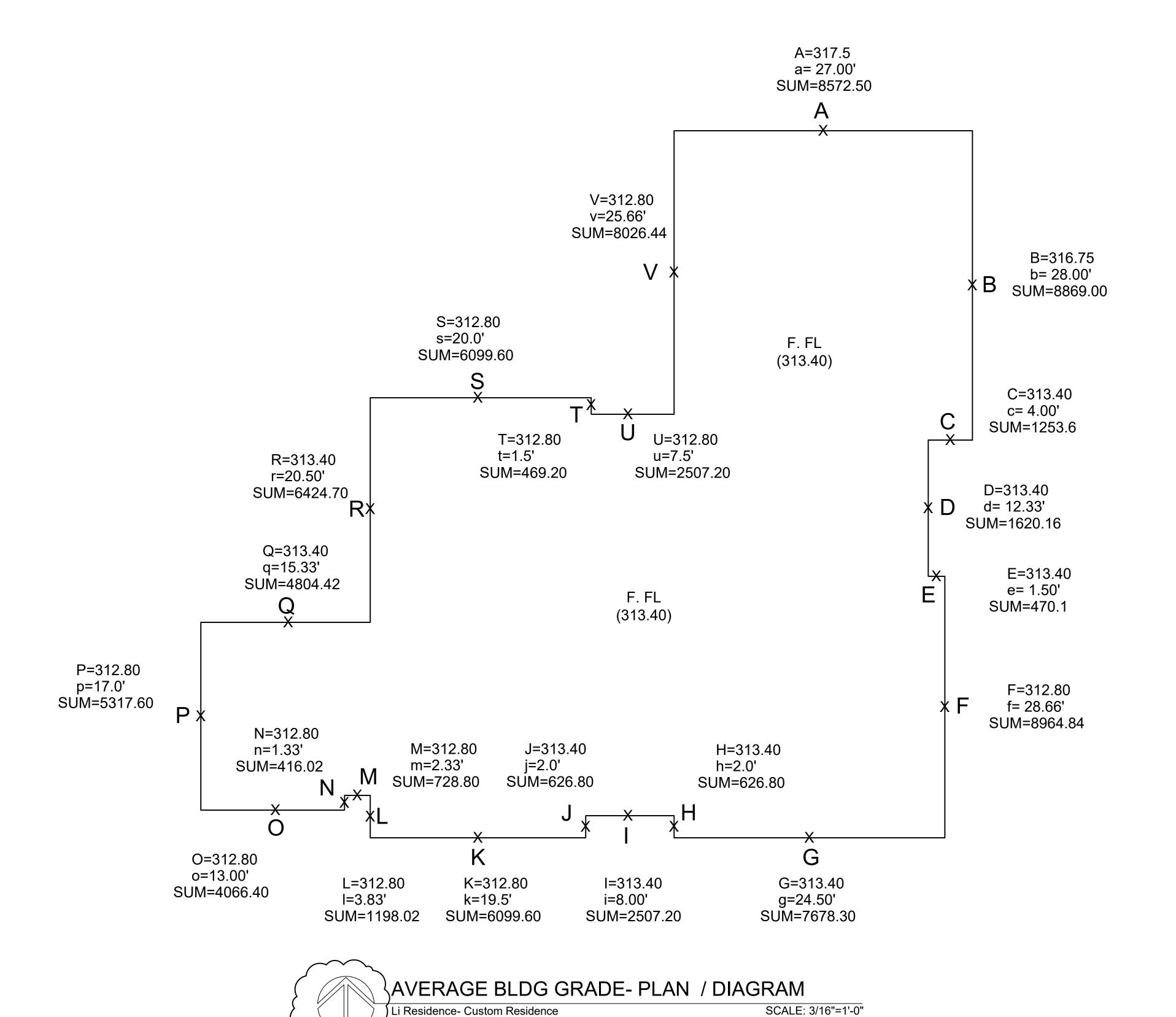


LI RESIDENCE
CUSTOM RESIDENCE
4657 86TH AVE. SE
MERCER ISLAND, WA 98040

AVERAGE BUILDING DIAGRAM / CALCS

DATE: 01-04- 2022 DESIGNED: JOB NO: 2022- 01

SHEET:



CHART

ELEV X	WALL	I FNGTH =	SUM
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SEGMENT	ELEV	WALL LENGTH	SUM
А	317.5	27.00'	8572.50
В	316.75	28.00'	8869.00
С	313.40	4.00'	1253.6
D	313.40	12.33'	3864.22
Е	313.40	1.5'	470.10
F	312.80	28.66'	8964.84
G	313.40	24.50'	7678.30
Н	313.40	2.0'	626.80
I	313.40	8.00'	2507.20
J	313.40	2.0'	626.80
K	312.80	19.5'	6099.60
L	312.80	3.83'	1198.02
М	312.80	2.33'	728.82
N	312.80	1.33'	416.02
0	312.80	13.0'	4066.40
Р	312.80	17.0'	5317.60
Q	313.40	15.33'	4804.42
R	313.40	20.50'	6424.70
S	312.80	20.0'	6256.00
Т	312.80	1.50'	469.20
U	312.80	7.5'	2346.00
V	312.80	25.66'	8026.44
TOTALS		285.47'	89606.58

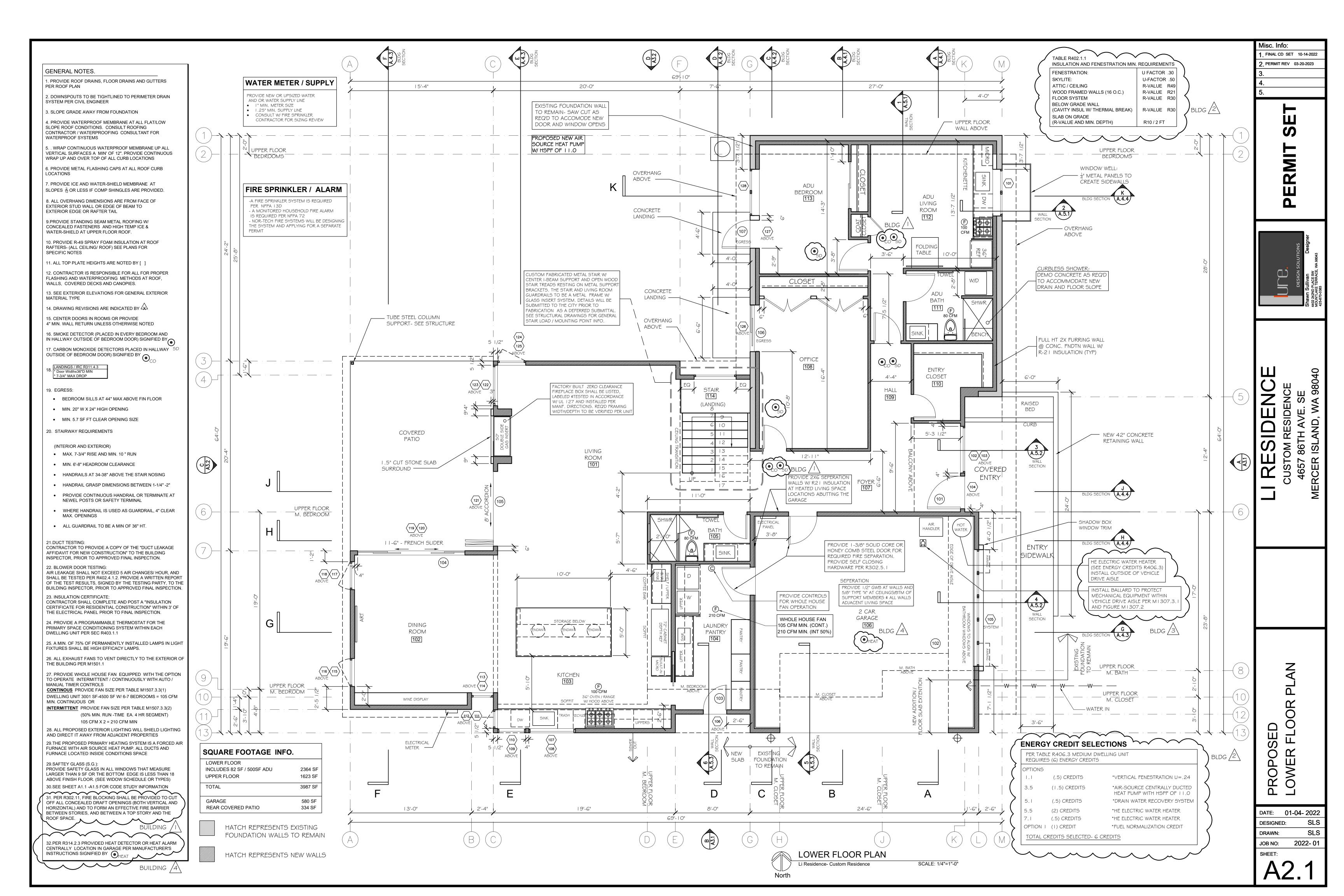
FORMULA(AVERAGE BUILDING ELEV)

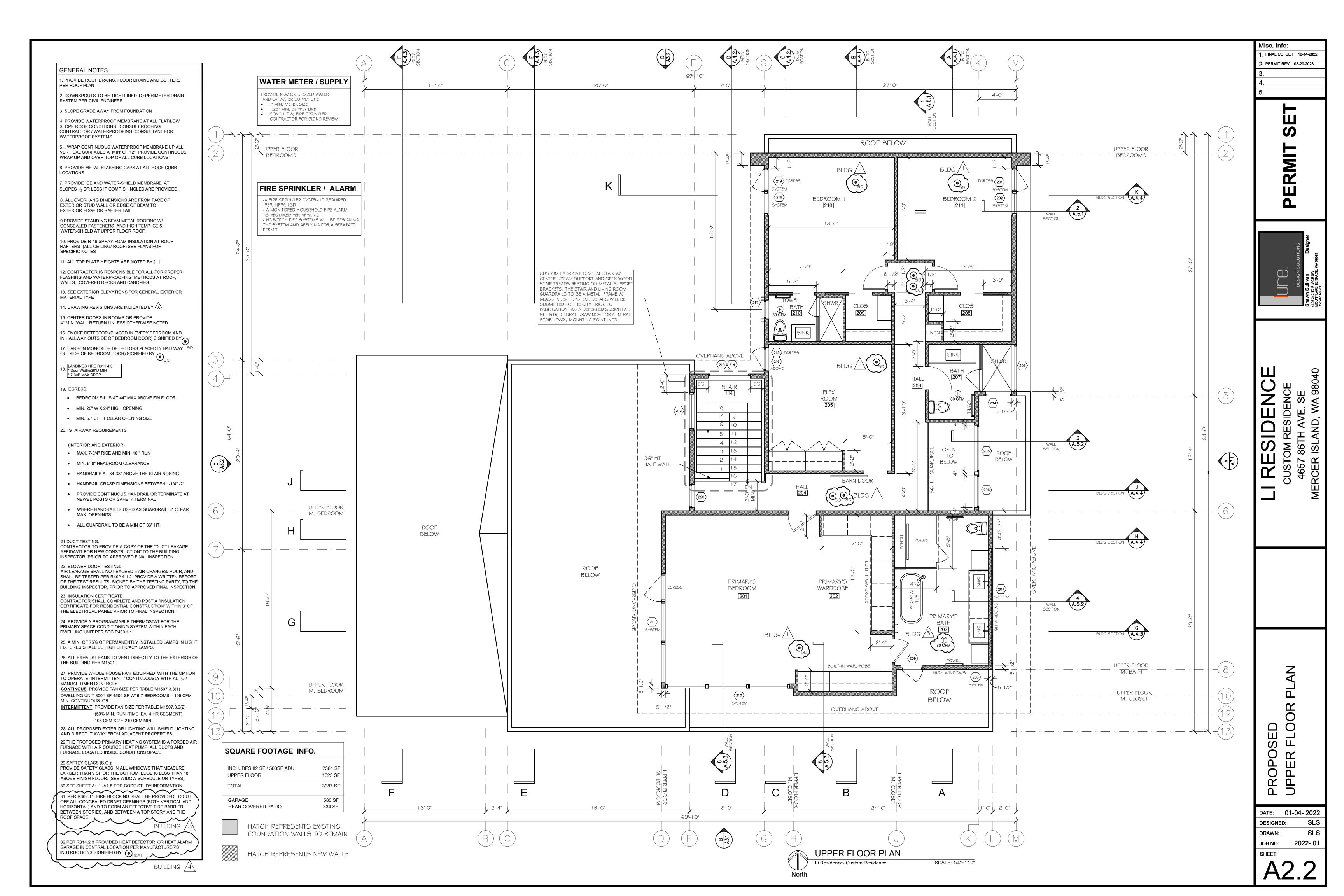
WEIGHTED SUM OF MID-POINT ELEVATION SUM OF WALL SEGMENT LENGTHS 89606.58 — = 313.89 (ABE) 285.47

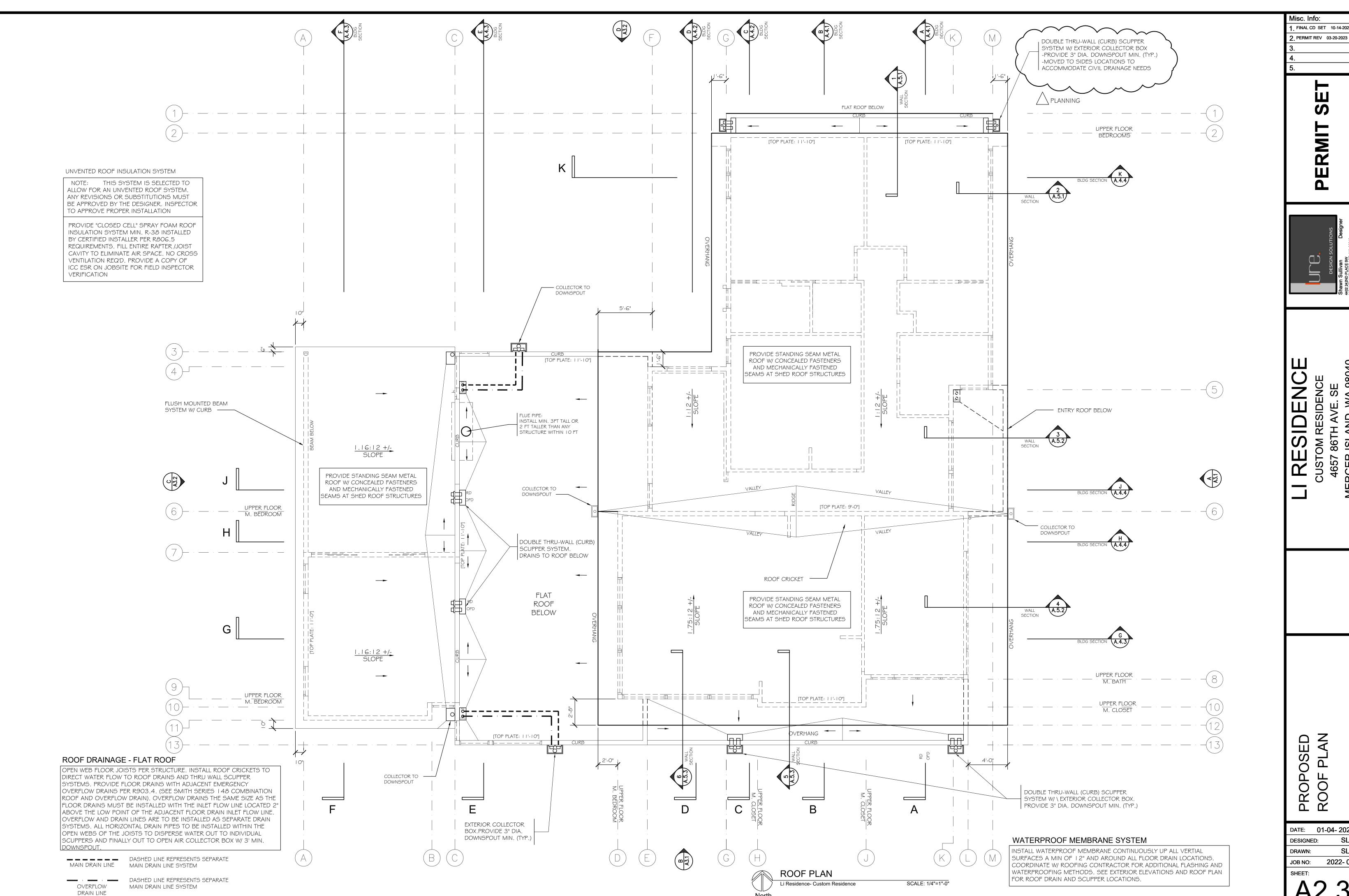
ALLOWABLE BUILDING HEIGHT MAX

ALLOWABLE BUILDIN HT + 30' ABOVE (ABE)

313.89 (ABE) + 30 = | ELEV 343.89

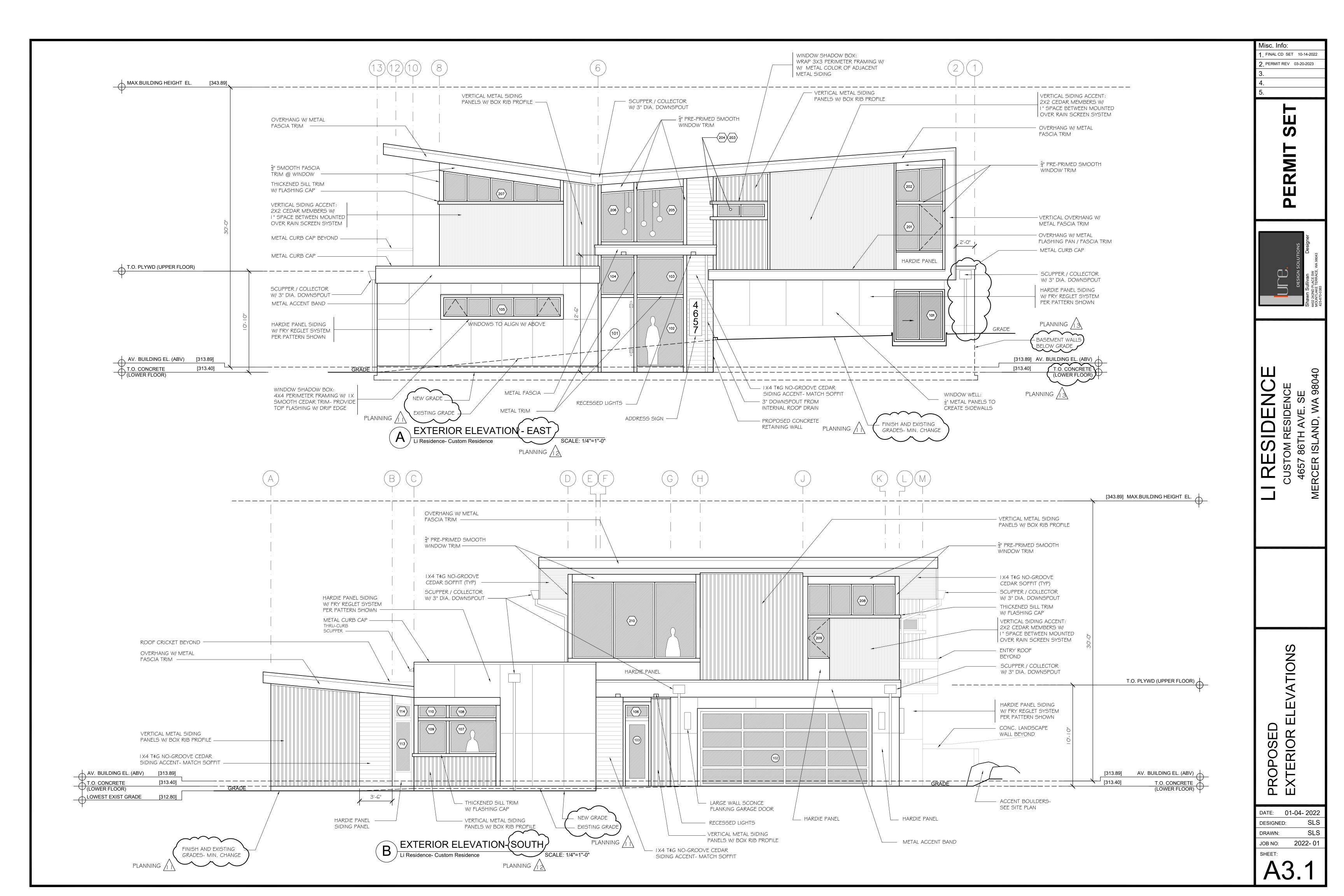


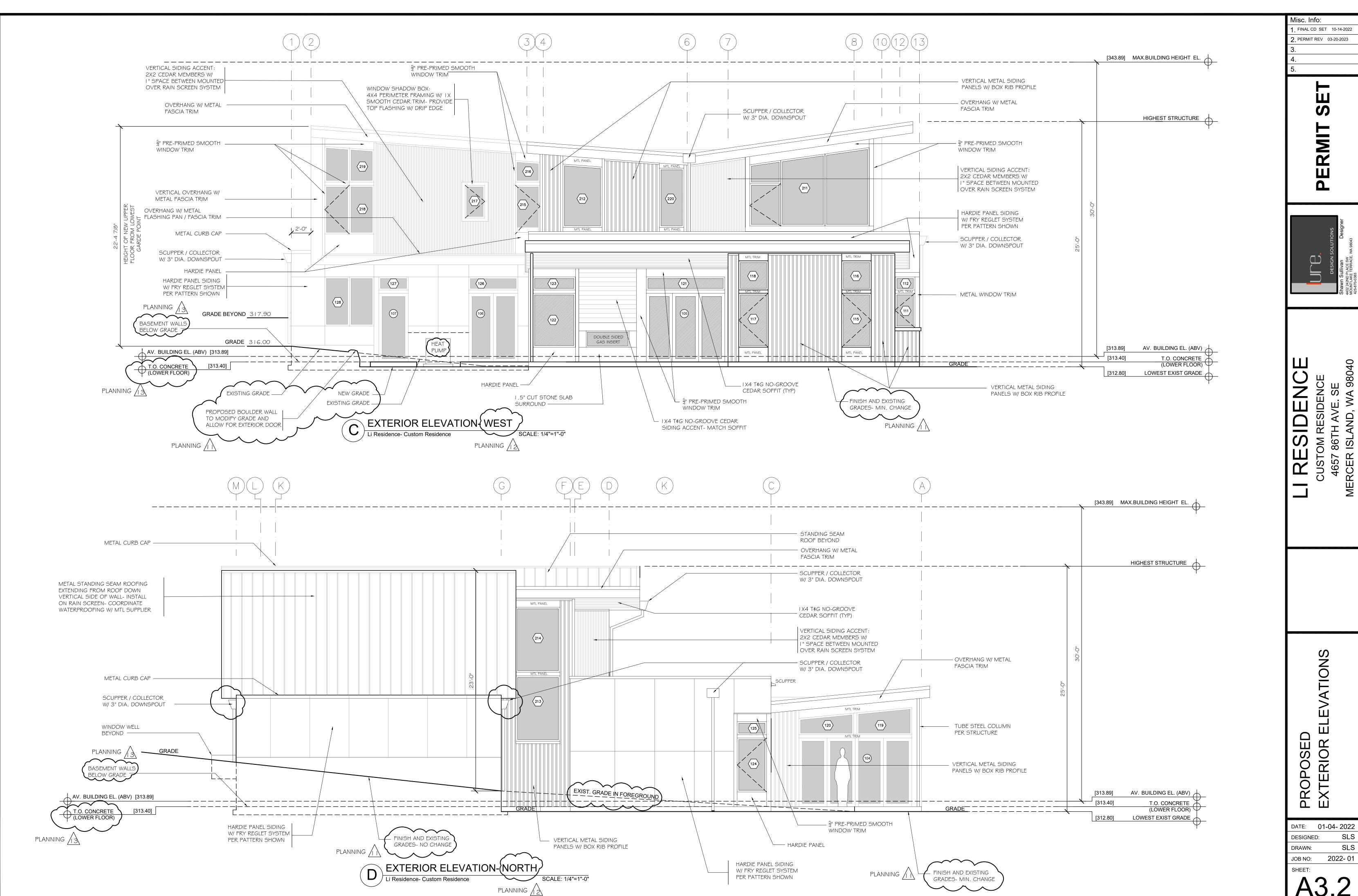


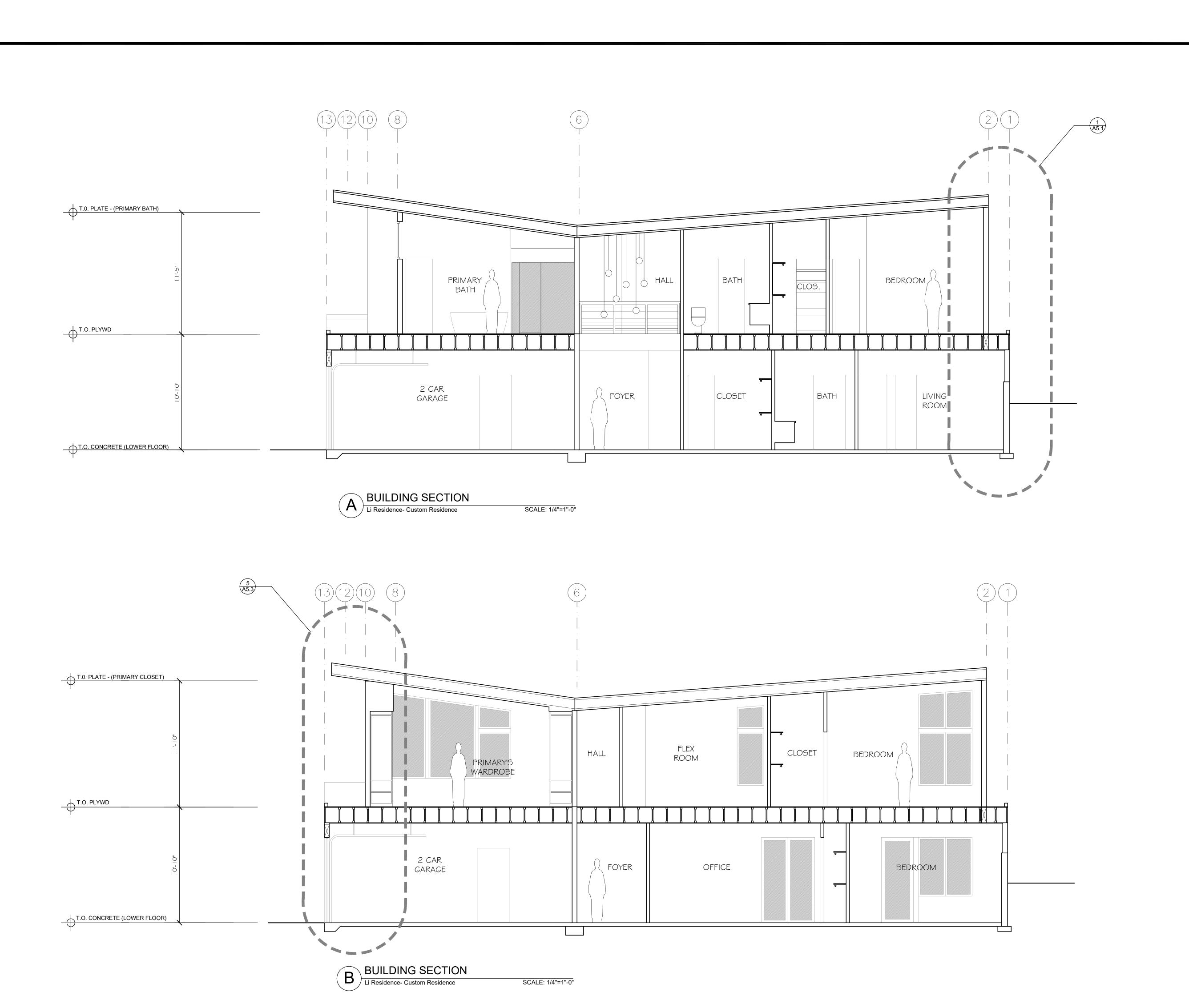


1 FINAL CD SET 10-14-2022 PERMIT REV 03-20-2023

DATE: 01-04- 2022 JOB NO: 2022- 01







Misc. Info:

1. FINAL CD SET 10-14-2022

2. PERMIT REV 03-20-2023

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PERMIT SE

DESIGN SOLUTIONS

Shawn Sullivan Designer

402 242ND PLACE SW
MOUNTLAKE TERRACE, WA 98043

CUSTOM RESIDENCE

OSED SING SECTIONS

 DATE:
 01-04- 2022

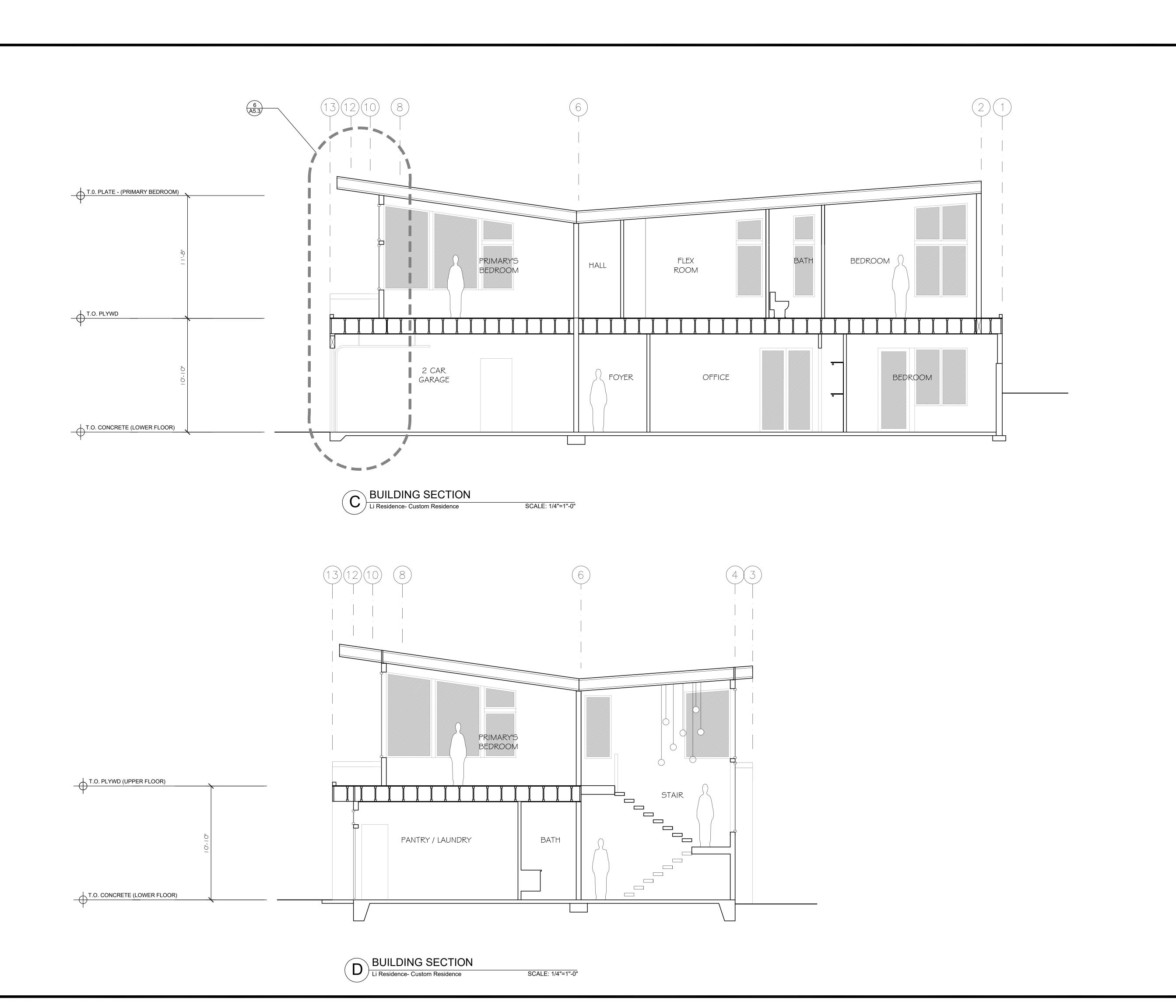
 DESIGNED:
 SLS

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 SLS

 JOB NO:
 2022- 01

 SHEET:

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Misc. Info:
1. FINAL CD SET 10-14-2022
2. PERMIT REV 03-20-2023
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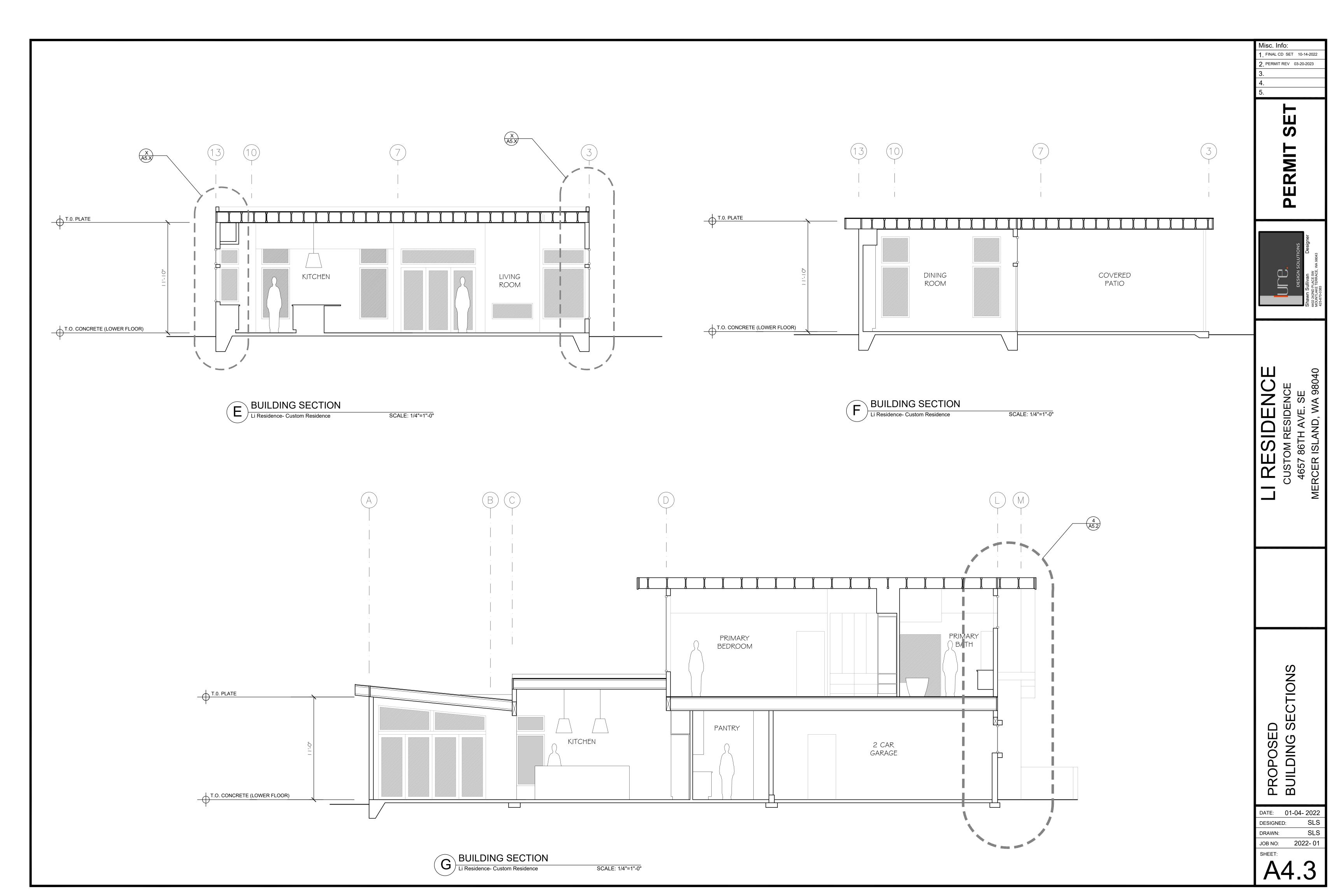
CUSTOM RESIDENCE

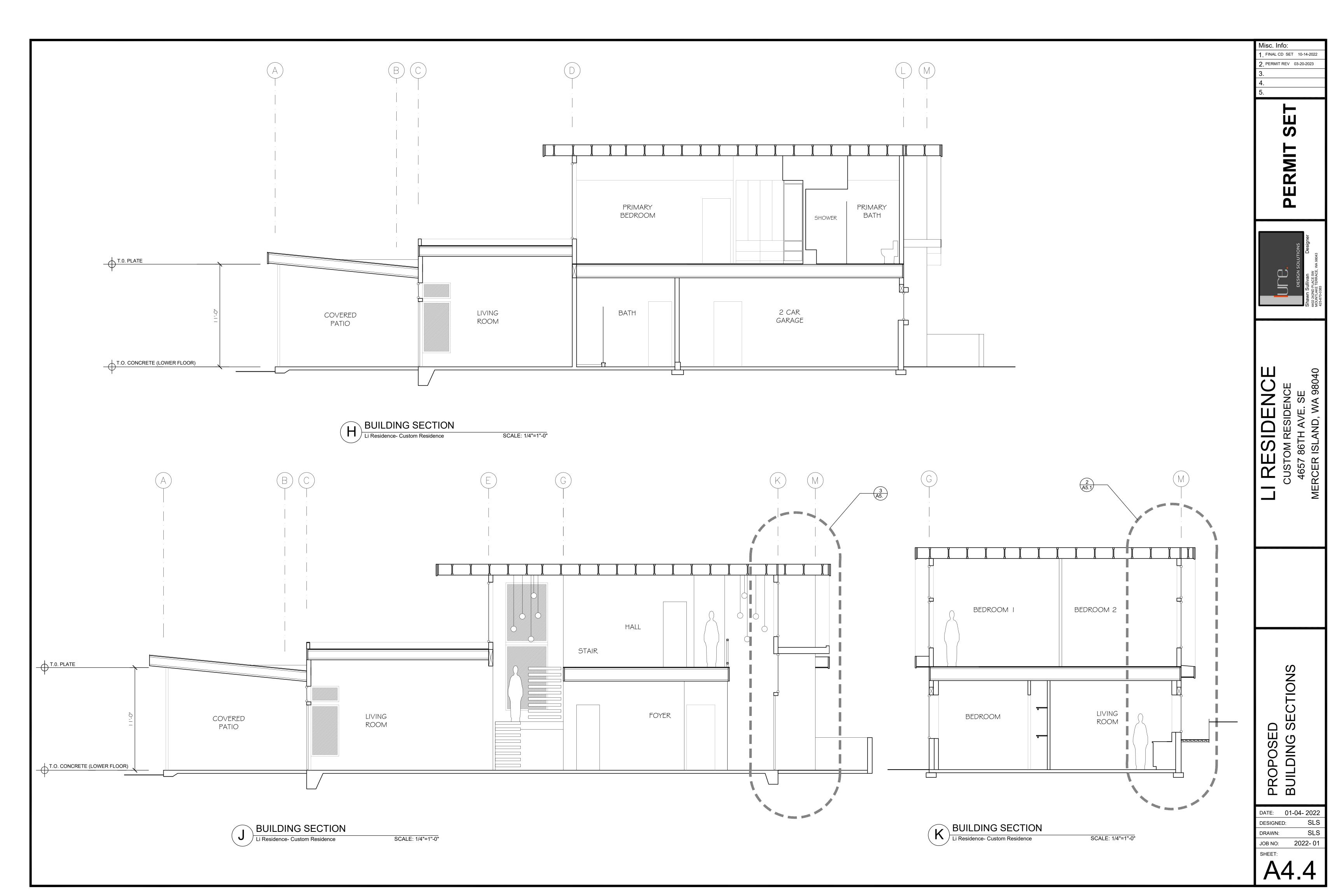
ECTIONS

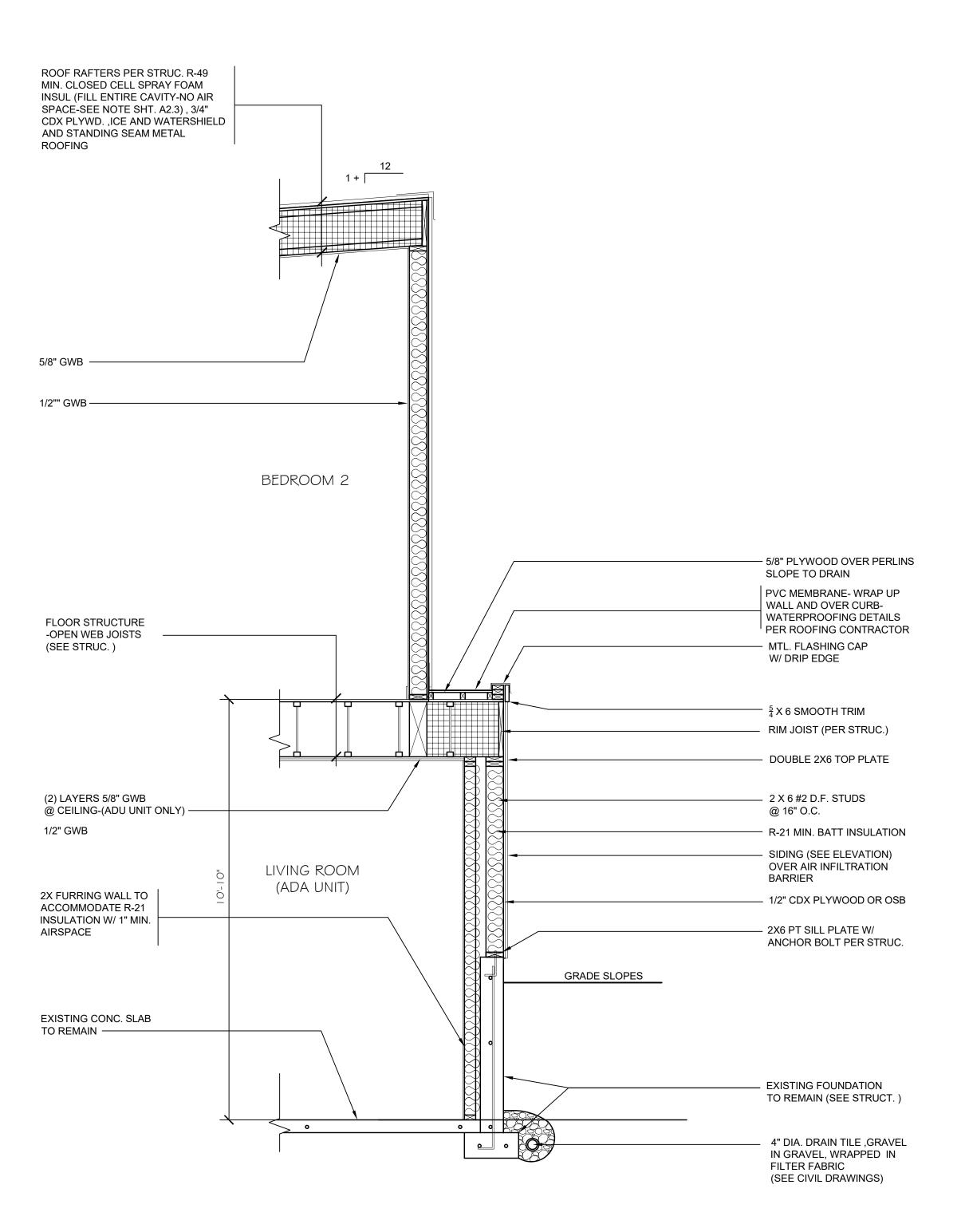
DATE: 01-04- 2022
DESIGNED: SLS
DRAWN: SLS

JOB NO: 2022- 01
SHEET:

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7



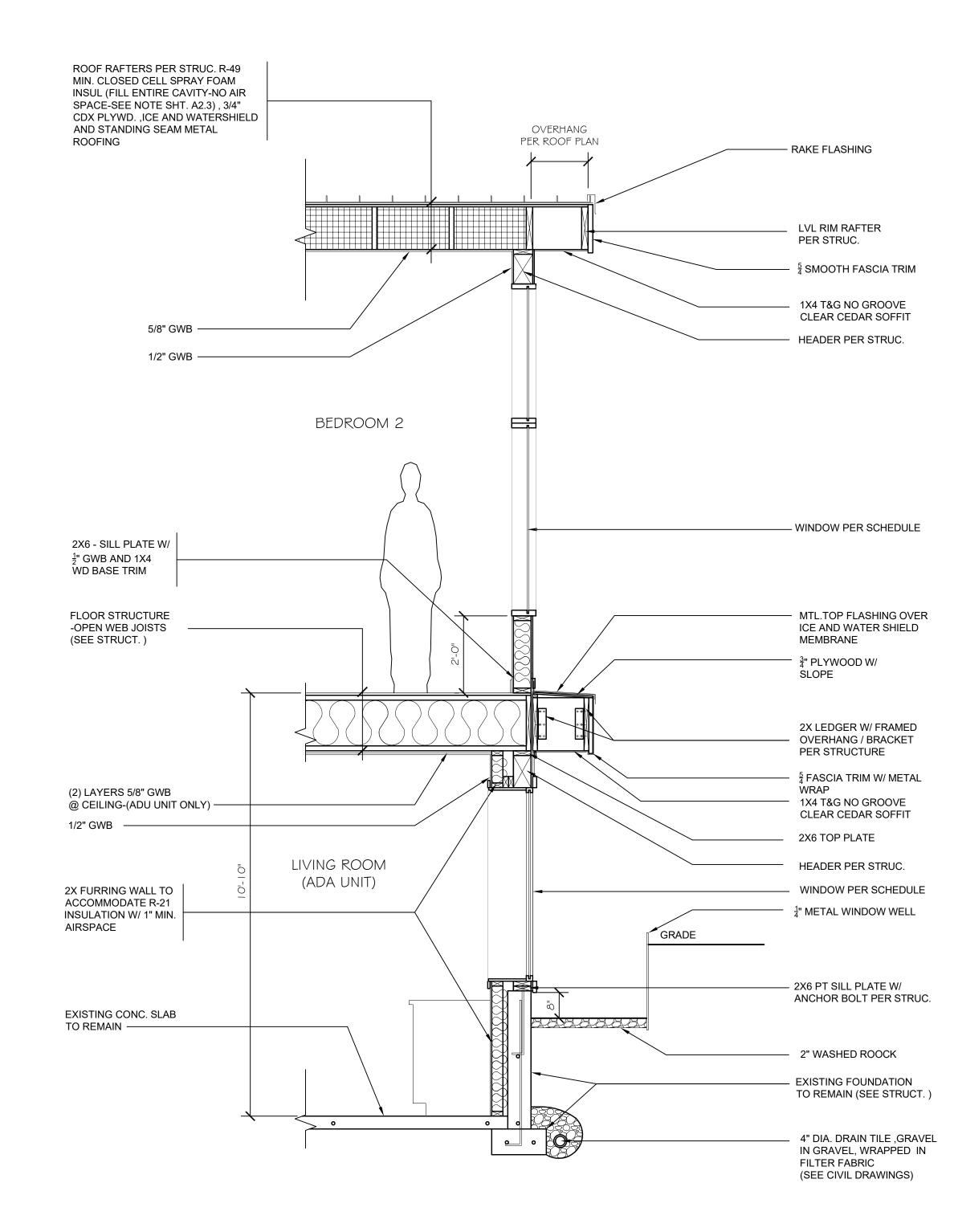




SCALE: 1/2"=1"-0"

WALL SECTION

Li Residence- Custom Residence



WALL SECTION

Li Residence- Custom Residence SCALE: 1/2"=1"-0"

Misc. Info:

1. FINAL CD SET 10-14-2022

2. PERMIT REV 03-20-2023

3. 4. 5.

T SET

DESIGN SOLUTIONS

awn Sullivan Designer
2.222ND PLACE SW
JMTLAKE TERRACE, WA 98043

ス**トレード プロース プローン** SUSTOM RESIDENCE 4657 86TH AVE. SE

PROPOSED WALL SECTIONS

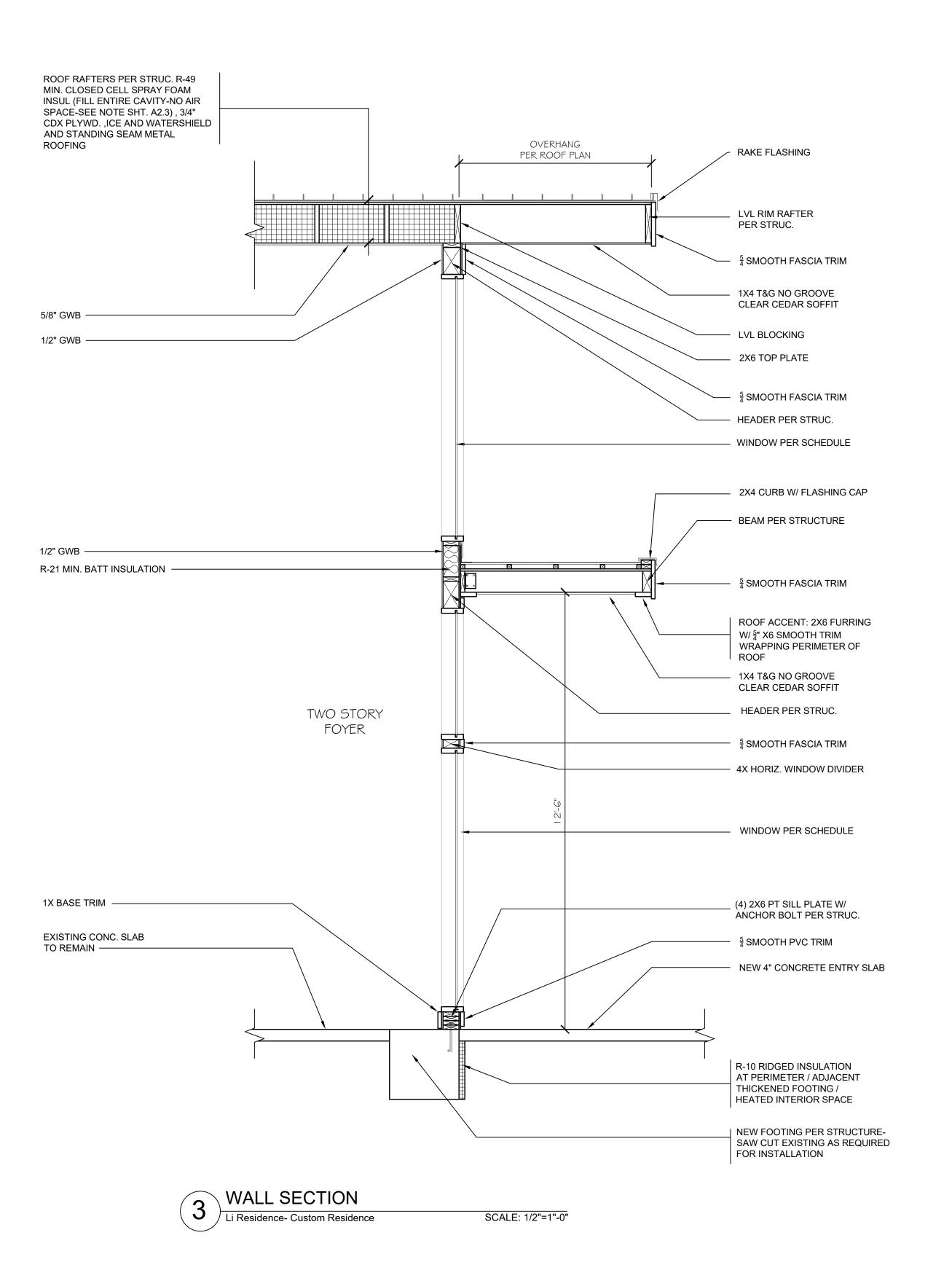
 DATE:
 01-04- 2022

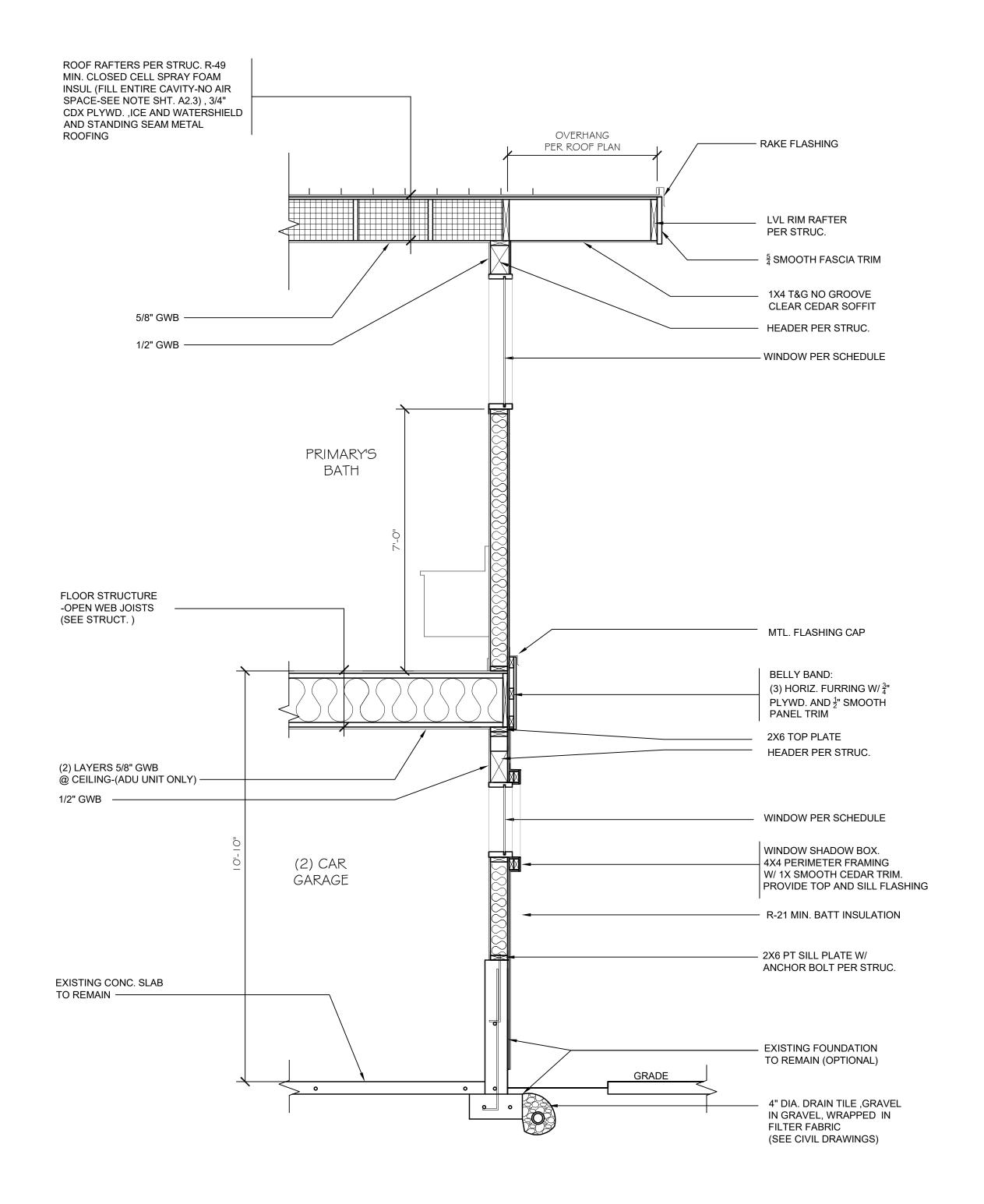
 DESIGNED:
 SLS

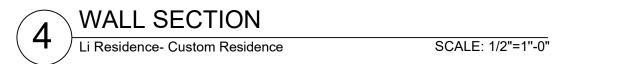
 DRAWN:
 SLS

 JOB NO:
 2022- 01

 SHEET:







Misc. Info:

1. FINAL CD SET 10-14-2022

2. PERMIT REV 03-20-2023

3.

4.

T SET

PERMIT
SIGN SOLUTIONS
an Designer
RACE WA 98043

LI RESIDENCE
CUSTOM RESIDENCE
4657 86TH AVE. SE
MERCER ISLAND, WA 98

PROPOSED WALL SECTIONS

 DATE:
 01-04- 2022

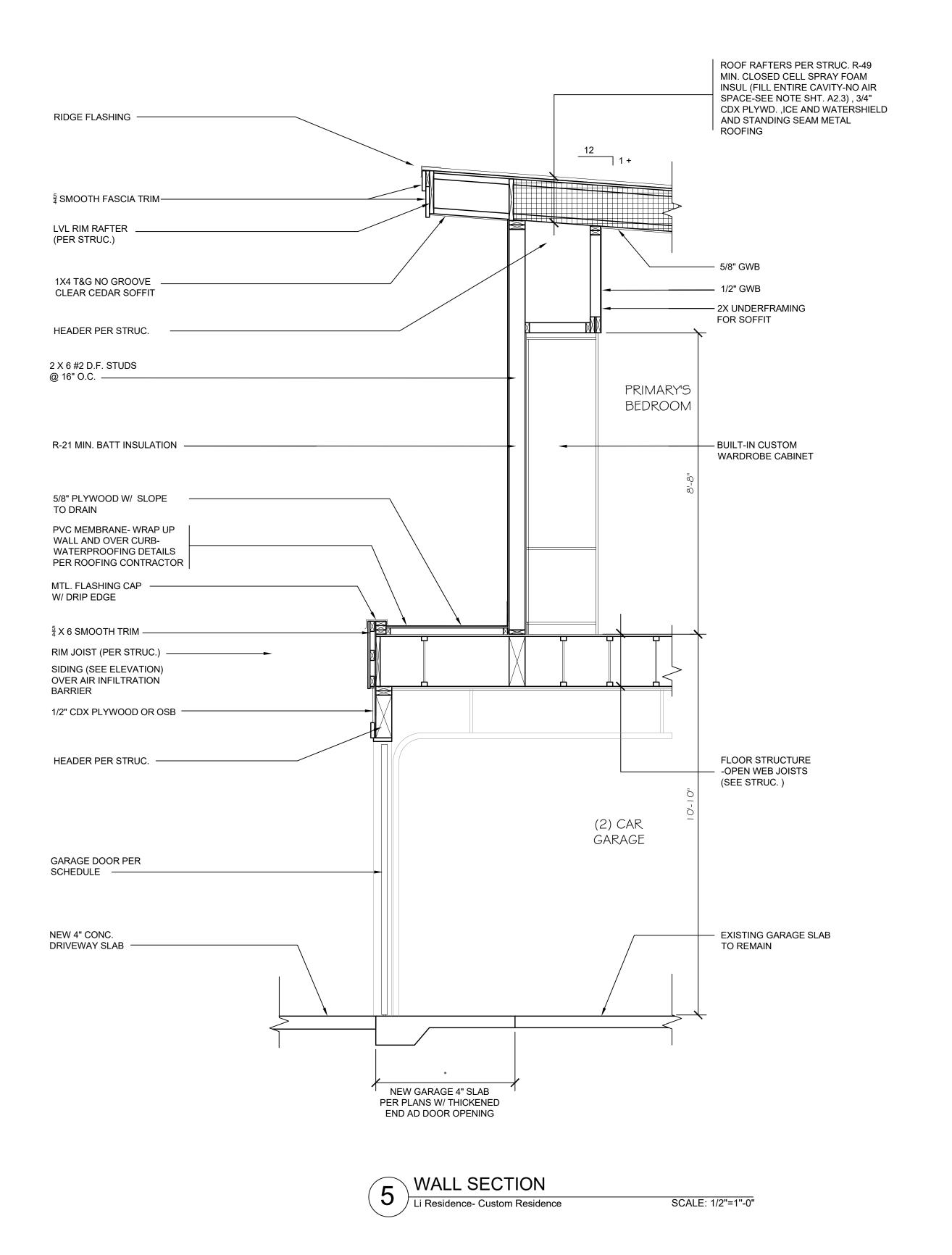
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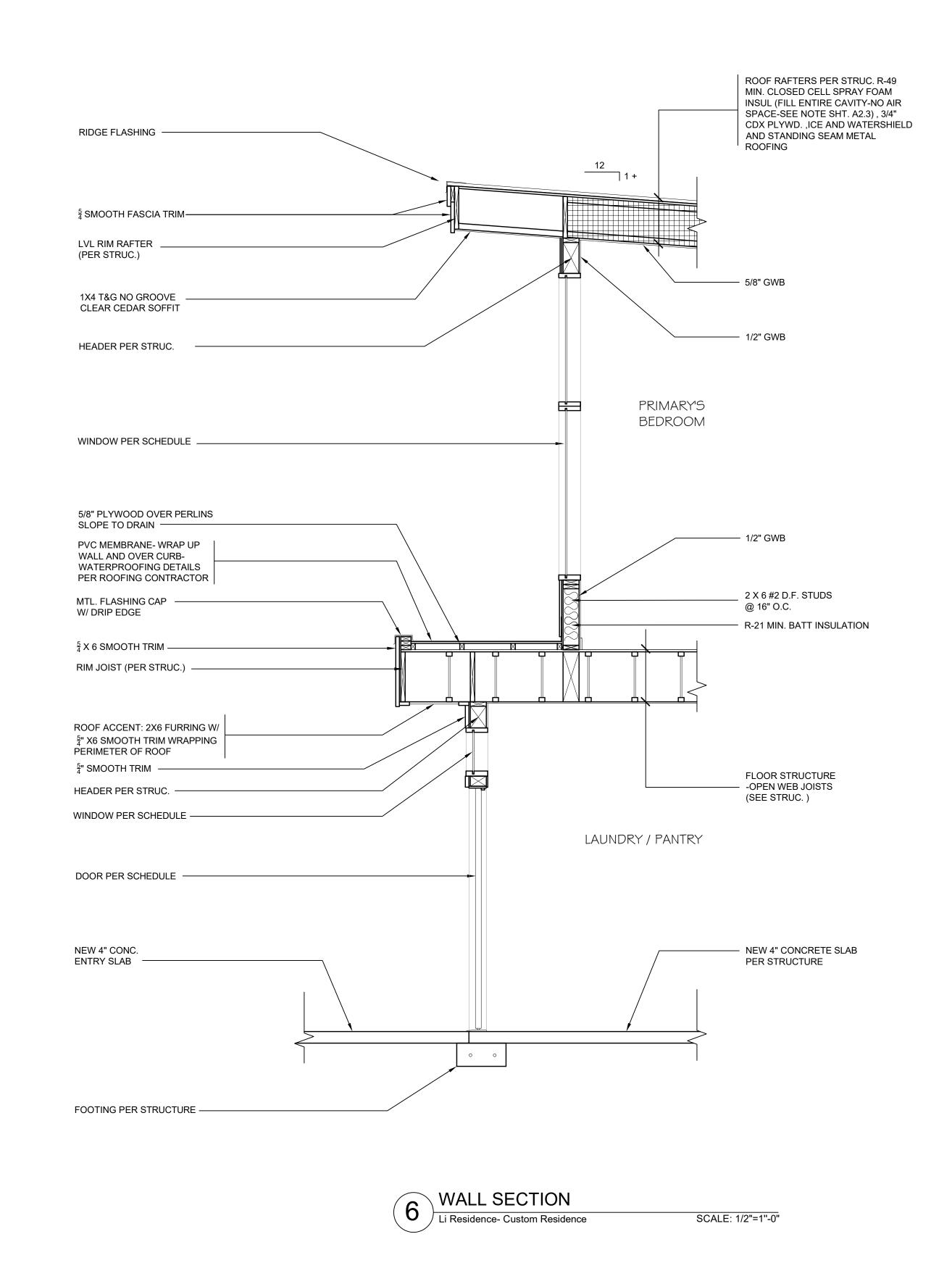
 DRAWN:
 SLS

 JOB NO:
 2022- 01

 SHEET:

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Misc. Info:

1. FINAL CD SET 10-14-2022

2. PERMIT REV 03-20-2023

3.

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5.

PERMIT Designer

Shawn Sullivan

402 242ND PLACE SW
MOUNTLAKE TERRACE, WA 98043

LI RESIDENCE
CUSTOM RESIDENCE
4657 86TH AVE. SE

ROPOSED VALL SECTIONS

DATE: 01-04- 2022
DESIGNED: SLS
DRAWN: SLS
JOB NO: 2022- 01
SHEET:

∩\//F	R FLOOR									
NDW NO.	ROOM NAME	R.O. SIZE W X H	MATERIAL	TYPE	SYSTEM / GROUP	STYLE	OPERATION	NOTES	U-FACTOR	
101	ADU- LIVING ROOM	5° X 5°	ALUMINUM	A	OTOTEW/ GROOT	SLIDER	OI LIVATION	SAFETY GLAZING	.24 MIN.	
102	FOYER	5° X 7 ⁴	ALUMINUM	В		PICTURE		SAFETY GLAZING	.24 MIN.	
103	FOYER	5° X 4°	ALUMINUM	В		PICTURE		SAFETY GLAZING	.24 MIN.	
104	FOYER	2 ⁶ X 1 ⁰	ALUMINUM	В		PICTURE		SAFETY GLAZING	.24 MIN.	
105	GARAGE	SEE WINDOW SYSTEM 105	ALUMINUM		SEE SYSTEM 105			SAFETY GLAZING	.24 MIN.	
106	LAUNDRY /PANTRY	2 ⁶ X 1 ⁴	ALUMINUM	В		PICTURE		SAFETY GLAZING	.24 MIN.	
107	KITCHEN	5° X 3°	ALUMINUM	В		PICTURE		SAFETY GLAZING	.24 MIN.	
108	KITCHEN	5° X 1 ⁴	ALUMINUM	В		PICTURE		SAFETY GLAZING	.24 MIN.	
109	KITCHEN	3° X 3°	ALUMINUM	В		PICTURE		SAFETY GLAZING	.24 MIN.	
110	KITCHEN	3° X 1 ⁴	ALUMINUM	В		PICTURE		SAFETY GLAZING	.24 MIN.	
111	KITCHEN	2° X 3 ⁶	ALUMINUM	В		PICTURE		SAFETY GLAZING	.24 MIN.	
112	KITCHEN	2° X 1 ⁴	ALUMINUM	С		CASEMENT	TBD	SAFETY GLAZING	.24 MIN.	
113	DINING ROOM	1 ⁹ X 5 ⁶	ALUMINUM	В		PICTURE		SAFETY GLAZING	.24 MIN.	
114	DINING ROOM	1 ⁹ X 1 ⁹ (SLOPED TOP)	ALUMINUM	D		PICTURE		SAFETY GLAZING	.24 MIN.	
115	DINING ROOM	3° X 5 ⁶	ALUMINUM	С		CASEMENT	TBD	SAFETY GLAZING	.24 MIN.	
116	DINING ROOM	3° X 3°	ALUMINUM	В		PICTURE		SAFETY GLAZING	.24 MIN.	
117	DINING ROOM	3° X 5 ⁶	ALUMINUM	С		CASEMENT	TBD	SAFETY GLAZING	.24 MIN.	
118	DINING ROOM	3° X 3°	ALUMINUM	В		PICTURE		SAFETY GLAZING	.24 MIN.	
119	DINING ROOM	5 ⁷ X 2 ⁹ (SLOPED TOP)	ALUMINUM	E		PICTURE		SAFETY GLAZING	.24 MIN.	
120	DINING ROOM	5 ⁷ X 2 ² (SLOPED TOP)	ALUMINUM	E		PICTURE		SAFETY GLAZING	.24 MIN.	
121	LIVING ROOM	8° X 1 ⁴	ALUMINUM	В		PICTURE		SAFETY GLAZING	.24 MIN.	
122	LIVING ROOM	4 ⁴ X 5 ⁶	ALUMINUM	В		PICTURE		SAFETY GLAZING	.24 MIN.	
123	LIVING ROOM	4 X 1 4	ALUMINUM	В		PICTURE		SAFETY GLAZING	.24 MIN.	
124	LIVING ROOM	3° X 5 ⁶	ALUMINUM	С		CASEMENT	TBD	SAFETY GLAZING	.24 MIN.	
125	LIVING ROOM	3° X 1 ⁴	ALUMINUM	В		PICTURE		SAFETY GLAZING	.24 MIN.	
126	OFFICE	5° X 1 ⁴	ALUMINUM	В		PICTURE		SAFETY GLAZING	.24 MIN.	
127	ADU-BEDROOM	3° X 1 ⁴	ALUMINUM	В		PICTURE		SAFETY GLAZING	.24 MIN.	
128	ADU-BEDROOM	5° X 5°	ALUMINUM	A		SLIDER		SAFETY GLAZING	.24 MIN.	

*VERIFY /MEASURE ALL R.O. FOR ACCURATE WINDOWS SIZES PRIOR TO ORDERING / MANUFACTURING

*WINDOW SIZES ABOVE REFLECT APPROXIMATE R.O. (ROUGH OPENINGS). WINDOWS TO BE SIZED ACCORDINGLY

* SEE PLANS AND ELEVATIONS FOR WINDOW TAG LOCATION XXXX

* SAFTEY GLAZING TO BE PROVIDE PER LOCAL CODE REQUIREMENTS

VNDW NO.	ROOM NAME	R.O. SIZE W X H	MATERIAL	TYPE	SYSTEM / GROUP	STYLE	OPERATION	NOTES	U-FACTOR	GLAZING ARE
201	BEDROOM	SEE WINDOW SYSTEM 201	ALUMINUM		SEE SYSTEM 201			SAFETY GLAZING	.24 MIN.	
202	BEDROOM	SEE WINDOW SYSTEM 202	ALUMINUM		SEE SYSTEM 202			SAFETY GLAZING	.24 MIN.	
203	BATHROOM	5° X 1 ⁴	ALUMINUM	A		SLIDER		SAFETY GLAZING	.24 MIN.	
204	BATHROOM	2 ¹⁰ X 1 ⁴	ALUMINUM	В		PICTURE		SAFETY GLAZING	.24 MIN.	
205	FOYER	5° X 6° (SLOPED TOP)	ALUMINUM	E		PICTURE		SAFETY GLAZING	.24 MIN.	
206	FOYER	3 ⁶ X 5 ⁶ (SLOPED TOP)	ALUMINUM	E		PICTURE		SAFETY GLAZING	.24 MIN.	
207	PRIMARY'S BATH	SEE WINDOW SYSTEM 207	ALUMINUM		SEE SYSTEM 207			SAFETY GLAZING	.24 MIN.	
208	PRIMARY'S BATH	SEE WINDOW SYSTEM 208	ALUMINUM		SEE SYSTEM 208			SAFETY GLAZING	.24 MIN.	
209	PRIMARY'S BATH	2 ⁴ X 4 ⁴	ALUMINUM	С		CASEMENT	TBD	SAFETY GLAZING	.24 MIN.	
210	PRIMARY'S BEDROOM	SEE WINDOW SYSTEM 210	ALUMINUM		SEE SYSTEM 210			SAFETY GLAZING	.24 MIN.	
211	PRIMARY'S BEDROOM	SEE WINDOW SYSTEM 211	ALUMINUM		SEE SYSTEM 211			SAFETY GLAZING	.24 MIN.	
212	STAIR	5° X 6 ⁸ (SLOPED TOP)	ALUMINUM	D		PICTURE		SAFETY GLAZING	.24 MIN.	
213	STAIR	4 ⁶ X 6 ⁸	ALUMINUM	В		PICTURE		SAFETY GLAZING	.24 MIN.	
214	STAIR	4 ⁶ X 6 ⁸	ALUMINUM	В		PICTURE		SAFETY GLAZING	.24 MIN.	
215	FLEX ROOM	2 ⁶ X 5 ⁰	ALUMINUM	С		CASEMENT	TBD	SAFETY GLAZING	.24 MIN.	
216	FLEX ROOM	2 ⁶ X 2 ² (SLOPED TOP)	ALUMINUM	D		PICTURE		SAFETY GLAZING	.24 MIN.	
217	BATHROOM	2 ⁰ X 3 ⁴	ALUMINUM	С		CASEMENT		SAFETY GLAZING	.24 MIN.	
218	BEDROOM	SEE WINDOW SYSTEM 218	ALUMINUM		SEE SYSTEM 218			SAFETY GLAZING	.24 MIN.	
219	BEDROOM	SEE WINDOW SYSTEM 219	ALUMINUM		SEE SYSTEM 219			SAFETY GLAZING	.24 MIN.	
220	STAIR	2 ⁶ X 6 ⁰	ALUMINUM	В		PICTURE		SAFETY GLAZING	.24 MIN.	
								SAFETY GLAZING	.24 MIN.	

*VERIFY /MEASURE ALL R.O. FOR ACCURATE WINDOWS SIZES PRIOR TO ORDERING / MANUFACTURING

*WINDOW SIZES ABOVE REFLECT APPROXIMATE R.O. (ROUGH OPENINGS). WINDOWS TO BE SIZED ACCORDINGLY

* SEE PLANS AND ELEVATIONS FOR WINDOW TAG LOCATION XXX

* SAFTEY GLAZING TO BE PROVIDE PER LOCAL CODE REQUIREMENTS

EXTE	RIOR DOO	R SCHEDULE	.)				
LOWE	R FLOOR						
WNDW NO.	ROOM NAME	R.O. SIZE W X H	MATERIAL	TYPE	OPERATION	NOTES	U-FACTOR
101	FOYER	3 ⁶ X 8 ⁰	ALUMINUM	A	LH SWING		
102	GARAGE	18° X 8°	ALUMINUM	С	OVER HEAD	(16) RELITES	.24 MIN.
103	LAUNDRY / PANTRY	2 ⁶ X 7 ⁰	ALUMINUM	В	RH SWING	FULL LITE	.24 MIN.
104	DINING ROOM	11 ⁶ X 7 ⁰	ALUMINUM	D	FENCH SLIDER	FULL LITE	.24 MIN.
105	LIVING ROOM	8° X 7°	ALUMINUM	E	ACCORDION	FULL LITE-(3) PANEL	.24 MIN.
106	OFFICE	5° X 7°	ALUMINUM	E	SLIDING DOOR	FULL LITE	.24 MIN.
107	ADU BEDROOM	3° × 7°	ALUMINUM	В	RH-R SWING	FULL LITE	.24 MIN.

Misc. Info:
1. FINAL CD SET 10-14-2022
2. PERMIT REV 03-20-2023
3.
4.

PERMIT SE

Shawn Sullivan Designer
MOUNTIANS
A402 242ND PLACE SW
MOUNTLAKE TERRACE, WA 98043
425-870-0333

CUSTOM RESIDENCE

OPOSED

ADOW AND EXTERIOR

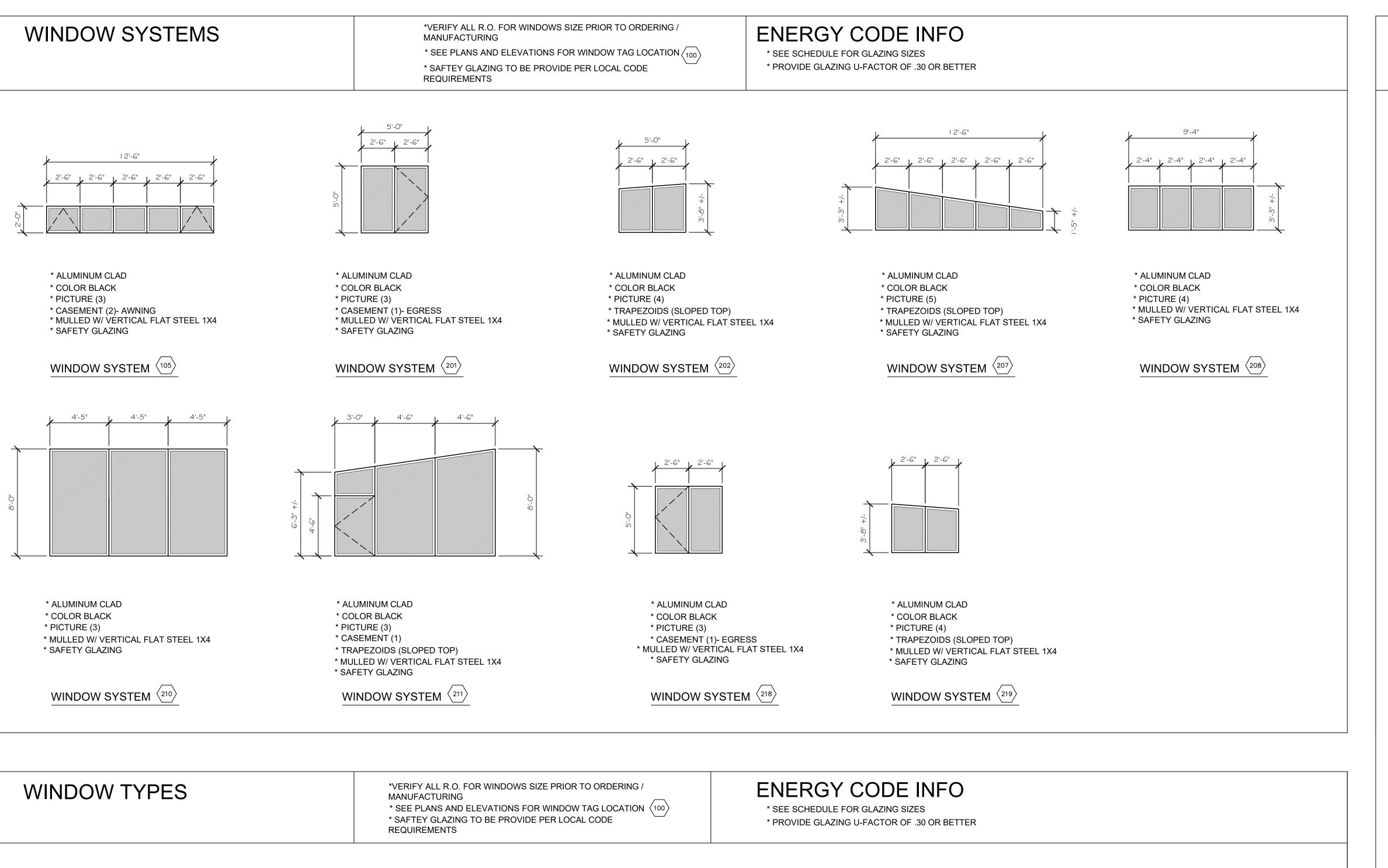
 DATE:
 01-04- 2022

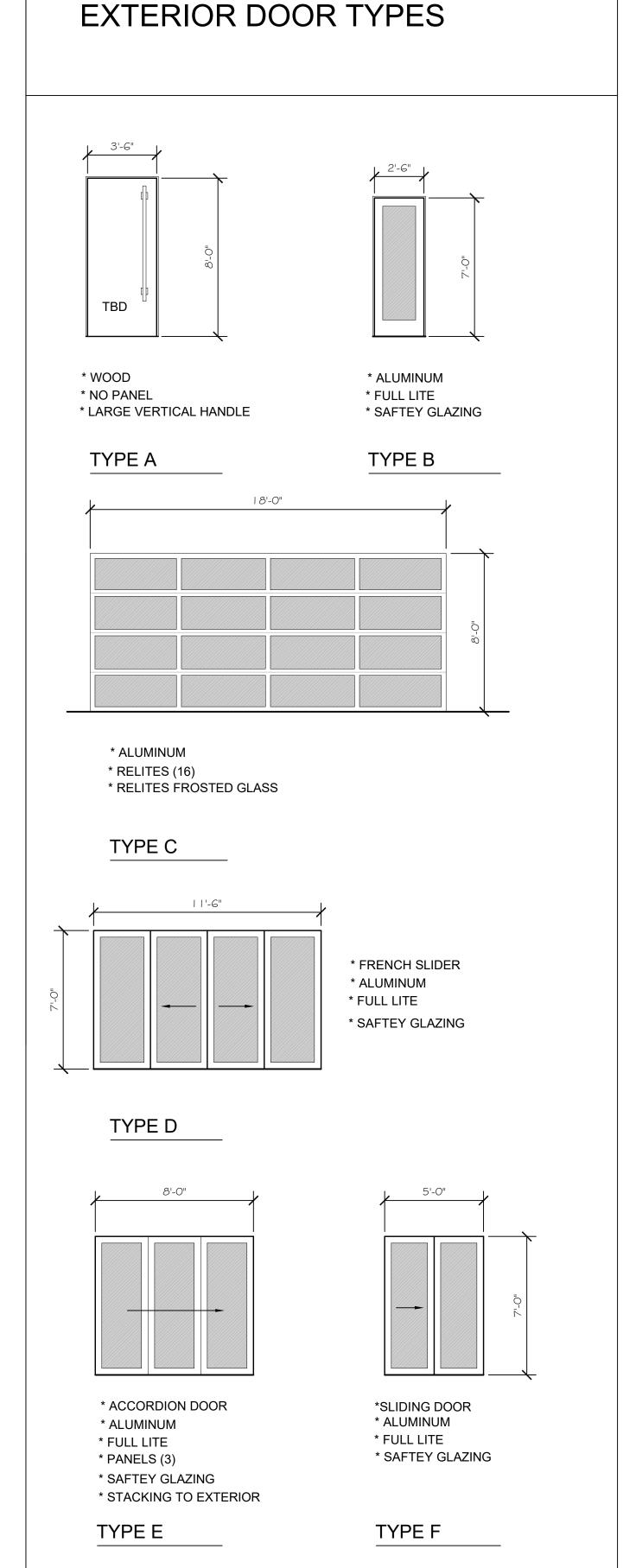
 DESIGNED:
 SLS

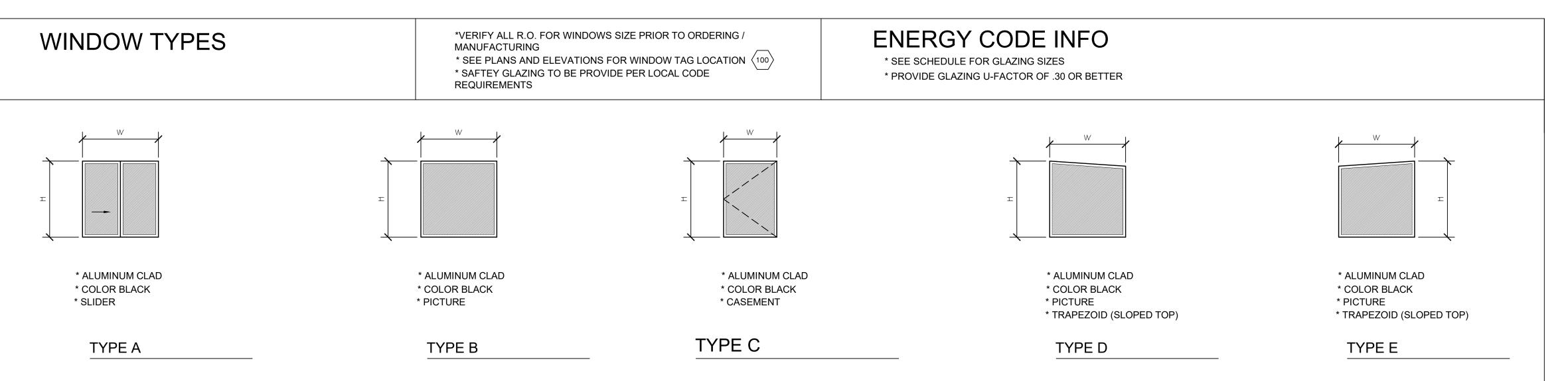
 DRAWN:
 SLS

 JOB NO:
 2022- 01

A6.1







DATE: 01-04- 2022
DESIGNED: SLS
DRAWN: SLS
JOB NO: 2022- 01
SHEET:

Misc. Info:

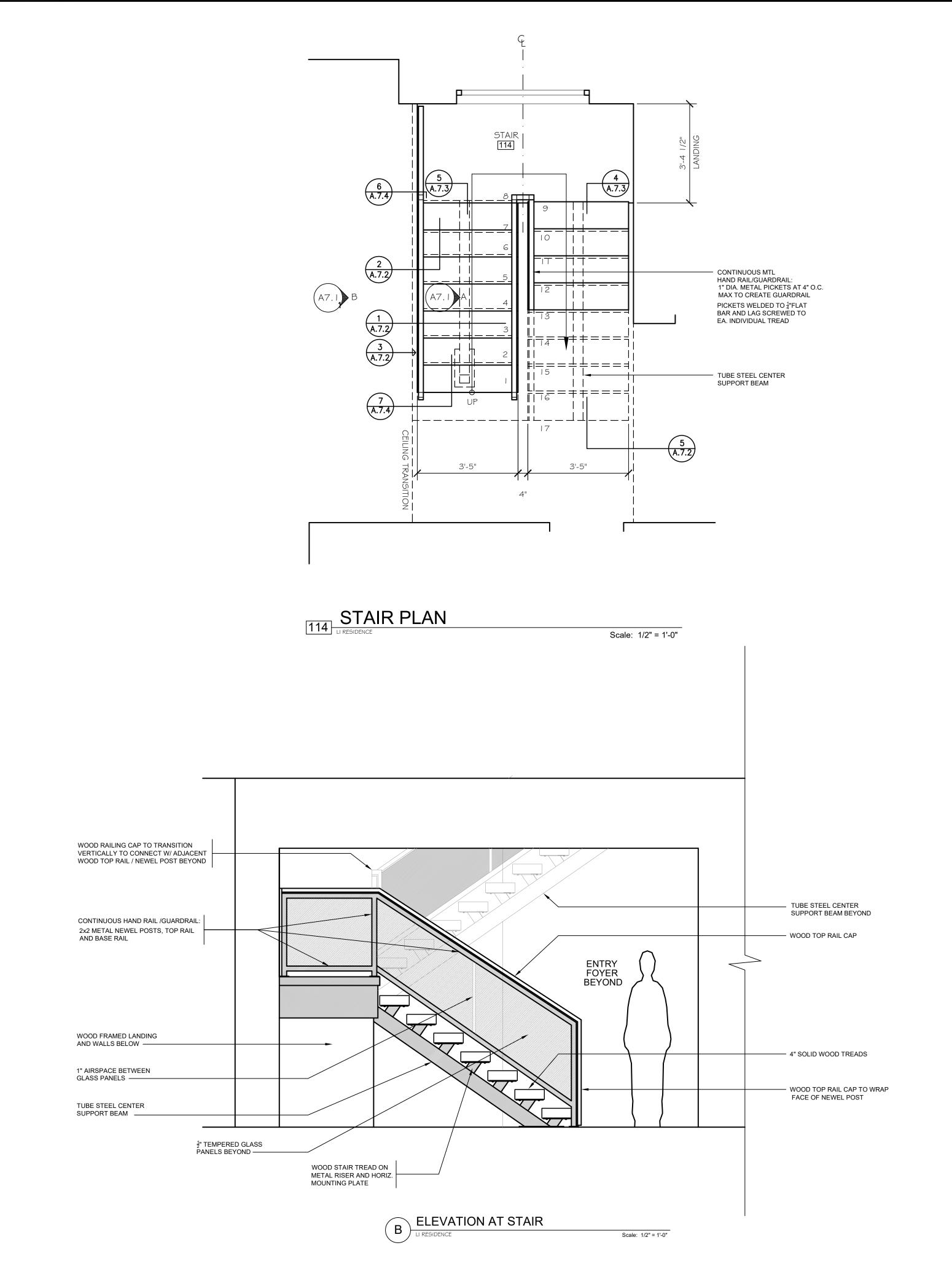
1. FINAL CD SET 10-14-2022 2. PERMIT REV 03-20-2023

PERMIT

SIDENC

LIRI CUS' 465 MERCE

A6.2



UPPER HALL BEYOND

TUBE STEEL CENTER SUPPORT BEAM BEYOND

WOOD TOP RAIL CAP

ENTRY FOYER BEYOND

SOLID WOOD TREADS

NOTE: (17) EQUAL RISERS @ 7-5/8" EACH. (7-3/4" MAX RISERS ALLOWERED)

ELEVATION AT STAIR - GAURDRAIL

Scale: 1/2" = 1'-0"

 WOOD TOP RAIL CAP TO WRAP FACE OF NEWEL POST

 $\frac{1}{2}$ " TEMPERED GLASS PANELS BEYOND ——

WOOD TOP RAIL CAP ----

WOOD RAILING CAP TO TRANSITION
VERTICALLY TO CONNECT W/ ADJACENT
WOOD TOP RAIL / NEWEL POST

CONTINUOUS HAND RAIL /GUARDRAIL: 2x2 METAL NEWEL POSTS, TOP RAIL AND BASE RAIL

1" AIRSPACE BETWEEN GLASS PANELS _____

TUBE STEEL CENTER SUPPORT BEAM ____

 $\frac{1}{2}$ " TEMPERED GLASS PANELS BEYOND ——

I RESIDENCE
CUSTOM RESIDENCE

Misc. Info:

1. FINAL CD SET 10-14-2022
2 PERMIT REV 03-20-2023

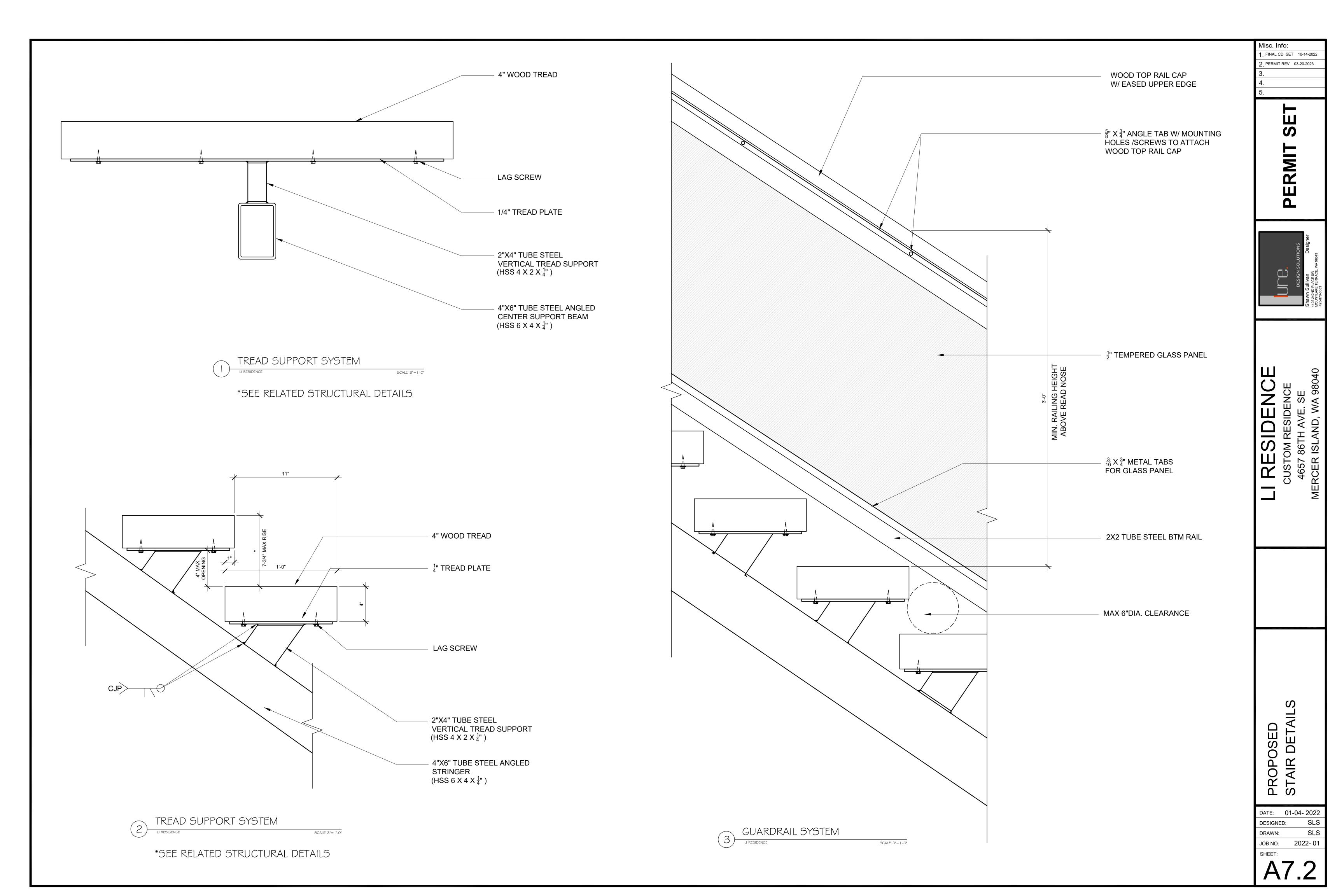
PERMIT

CUSTOM RESIDEN
4657 86TH AVE. SI
MERCER ISLAND, WA

PROPOSED STAIR PLANS AND FI VATIONS

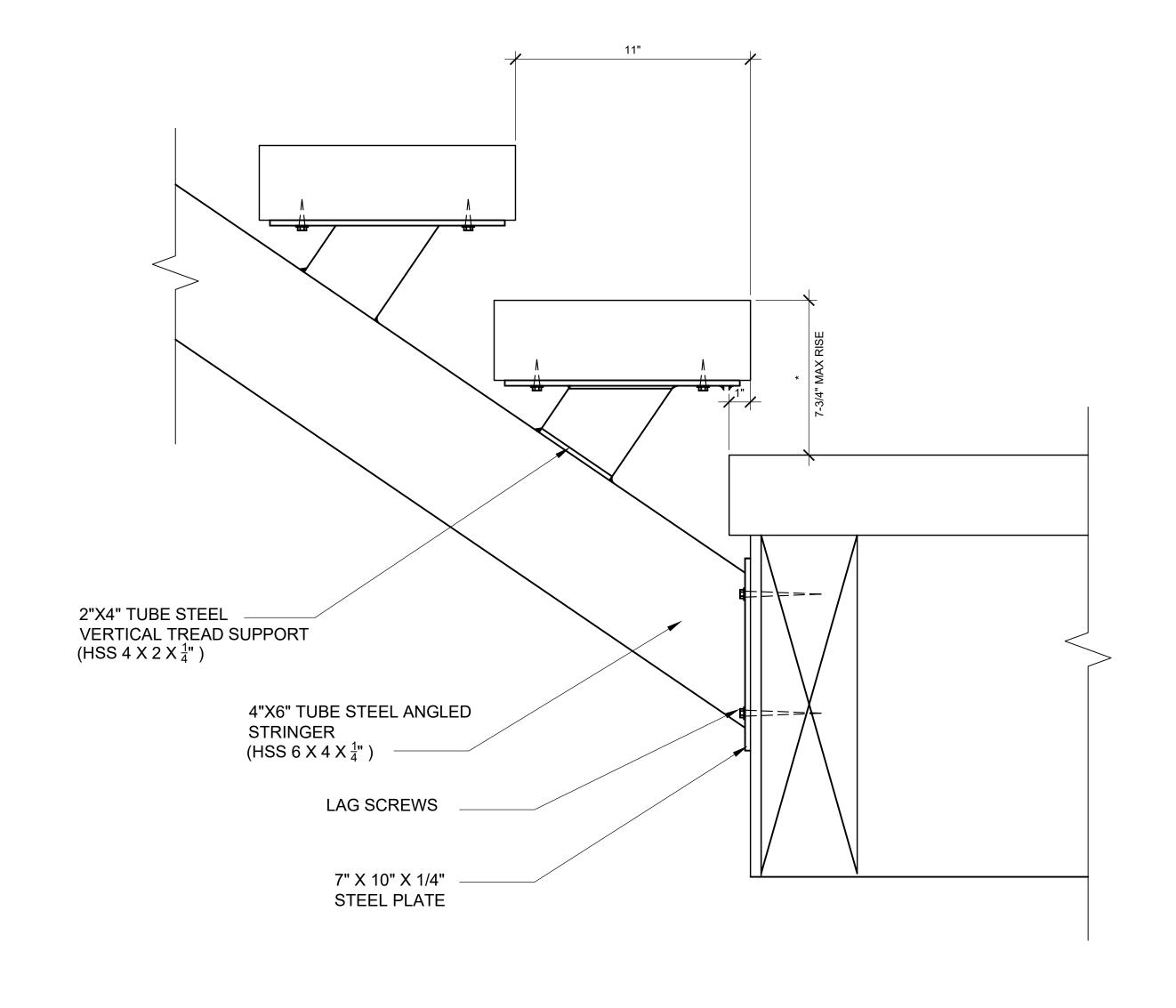
DATE: 01-04- 2022
DESIGNED: SLS
DRAWN: SLS
JOB NO: 2022- 01
SHEET:

SHEET:
A7.



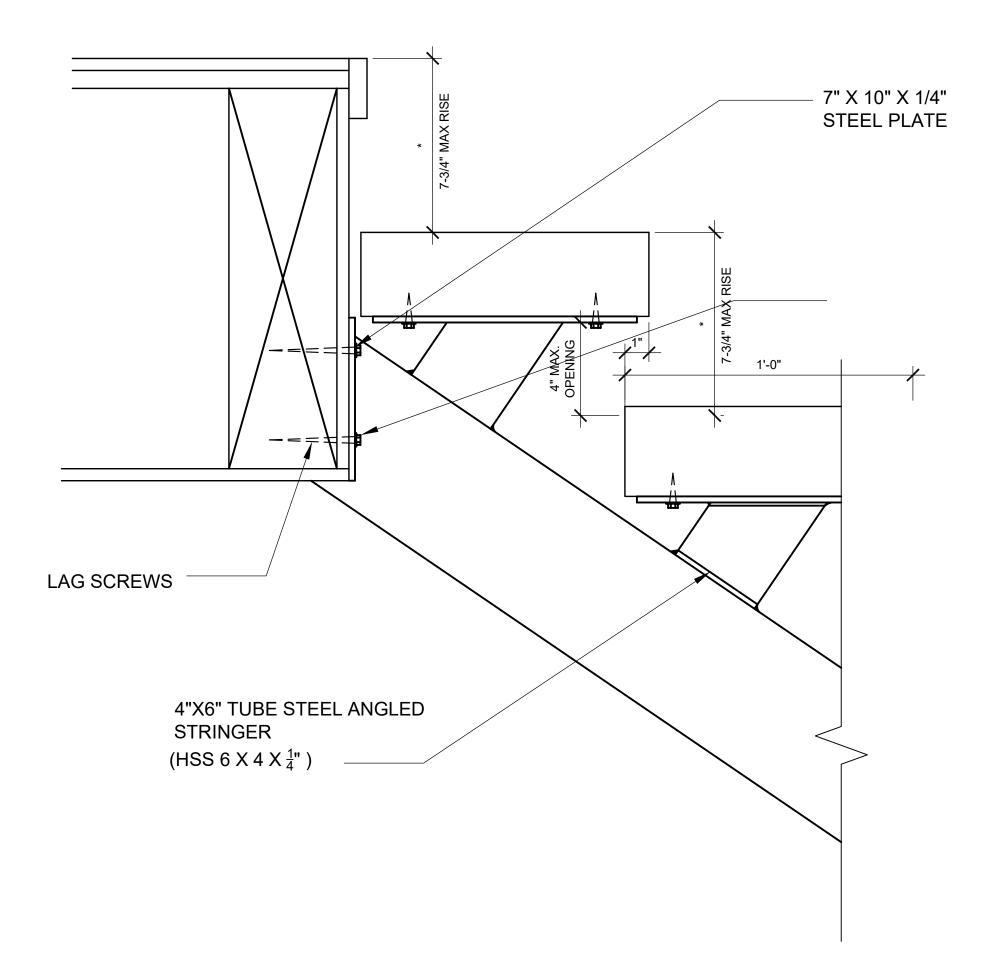
DATE: 01-04- 2022 DESIGNED: DRAWN: JOB NO: 2022- 01

SLS SHEET:



STAIR LANDING CONNECTION SCALE' 3"= 1'-0"

*SEE RELATED STRUCTURAL DETAILS



5 STAIR LANDING CONNECTION

LI RESIDENCE SCALE' 3"= 1'-0"

*SEE RELATED STRUCTURAL DETAILS

Misc. Info:

1 FINAL CD SET 10-14-2022

DATE: 01-04- 2022 SLS DESIGNED: DRAWN: JOB NO: 2022- 01 SHEET:

10" X 15" X 1/4" STEEL PLATE 4"X6" TUBE STEEL ANGLED STRINGER (HSS 6 X 4 X $\frac{1}{4}$ ") 4" CONCRETE SLAB (6) $\frac{1}{2}$ "DIA. X 4" EXPANSION ANCHOR BOLTS BASE ATTACHMENT TO CONCRETE SLAB

*SEE RELATED STRUCTURAL DETAILS

GUARDRAIL / NEWEL POST

LANDING

WOOD TOP RAIL CAP W/ EASED UPPER EDGE

WOOD TOP RAIL CAP

WELD

TOP SIDE

 $\frac{3}{16}$ X $\frac{3}{4}$ " METAL TABS FOR GLASS PANEL

POST BEYOND

2X2 TUBE STEEL NEWEL

1" TEMPERED GLASS PANEL

2X2 TUBE STEEL BTM RAIL

MOUNTING TAB (2) 3/8"

X 2.5" ALLEN HEÀD LAG

 $\frac{3}{16}$ X $\frac{3}{4}$ " METAL TABS FOR GLASS PANEL

WELD -

WELD -

SCREWS

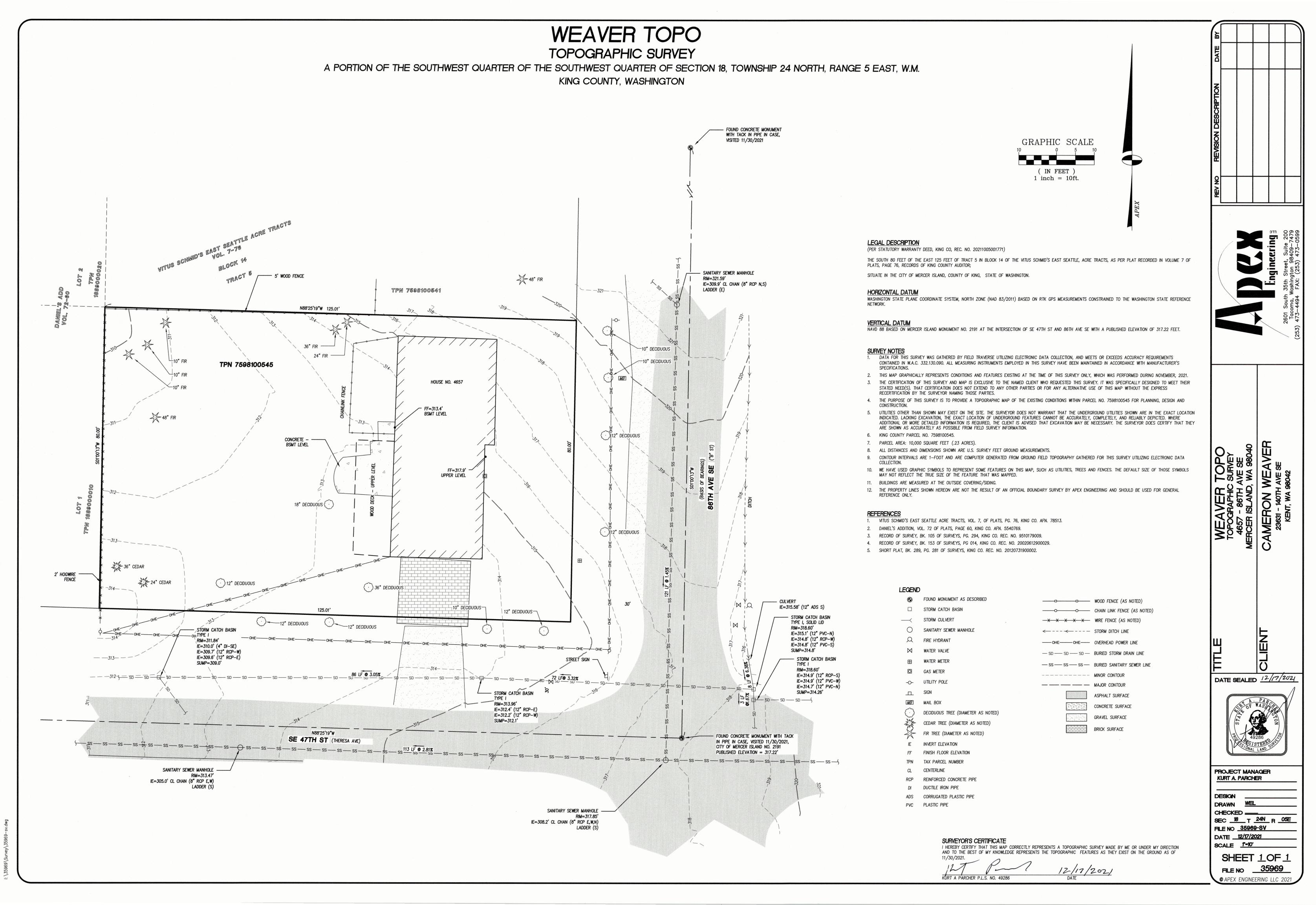
WELD

 $\frac{5}{8}$ " X $\frac{3}{4}$ " ANGLE TAB W/ MOUNTING

HOLES /SCREWS TO ATTACH

5/8" X 2" METAL RAIL EA. SIDE -REMAINING SPACE BETWEEN TO ALLOW GLASS PANEL TO BE INSERTED INSERTED FROM

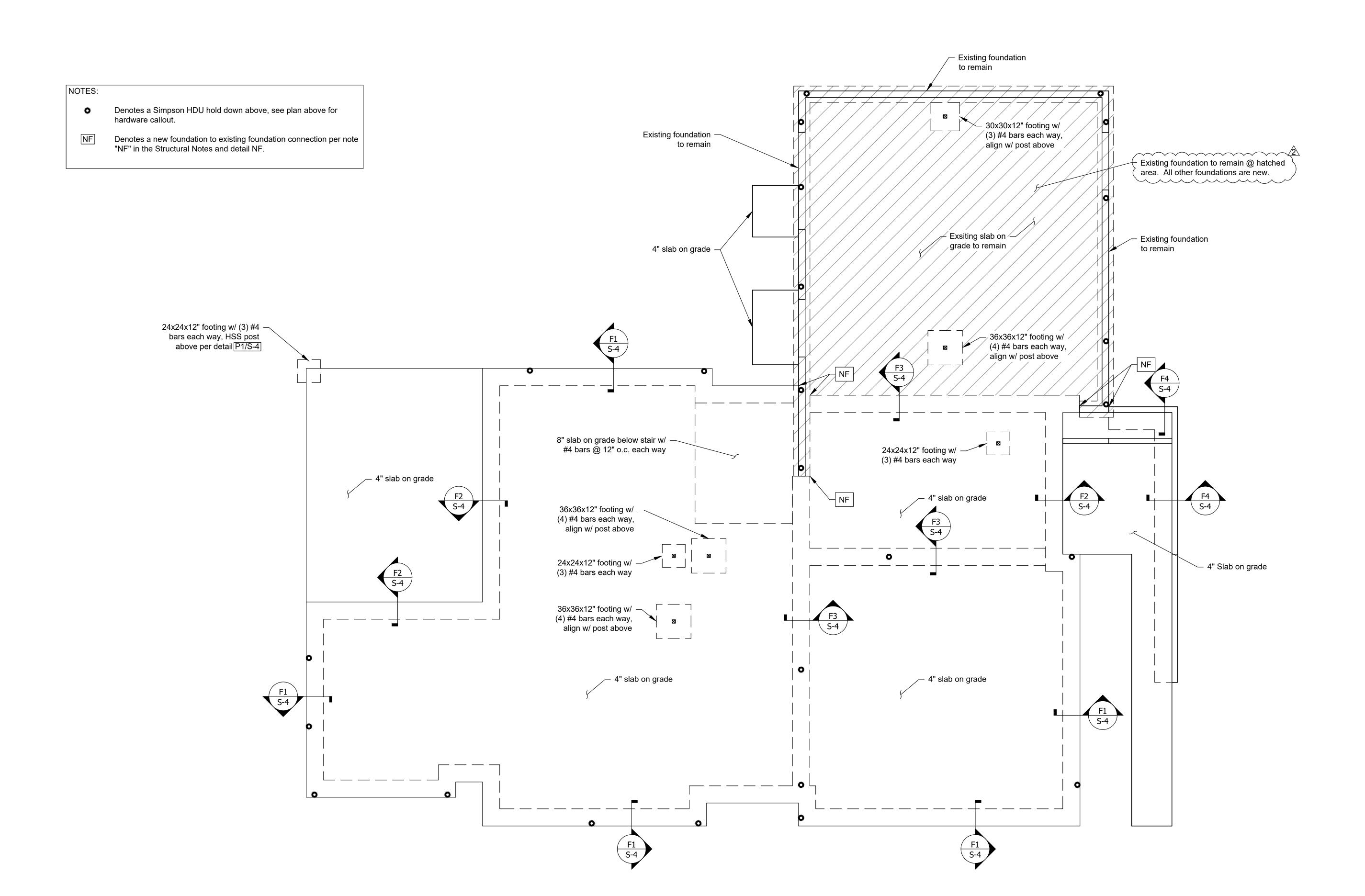
*SEE RELATED STRUCTURAL DETAILS



040

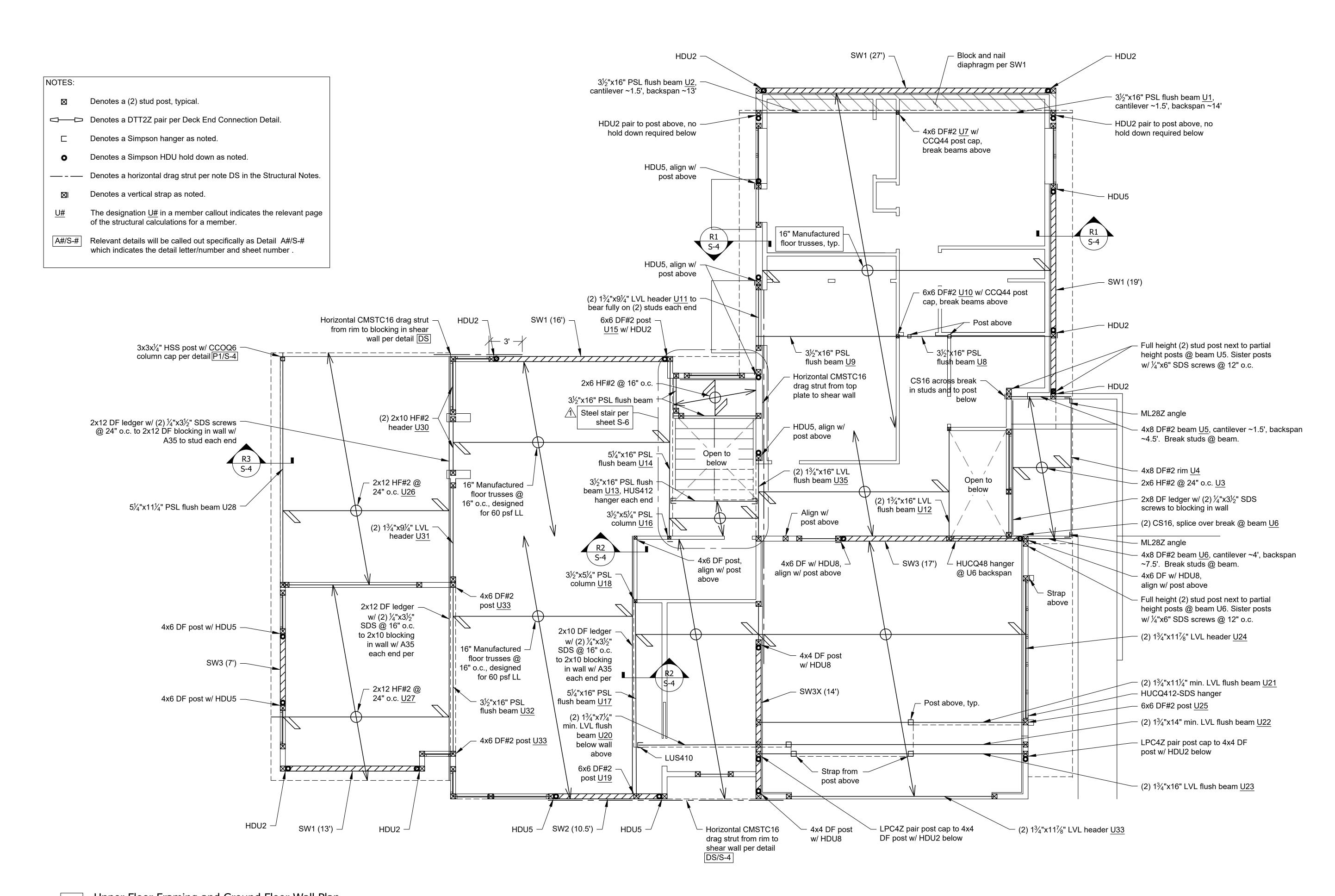
Scale: 1/4" = 1'-0"

S-1



Scale: 1/4" = 1'-0"

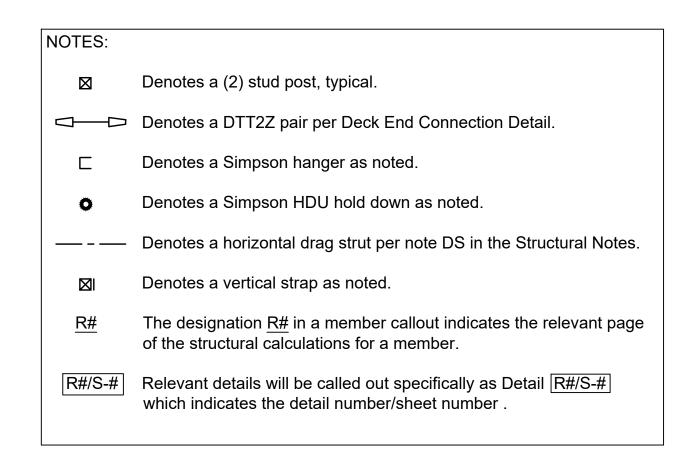
S-2

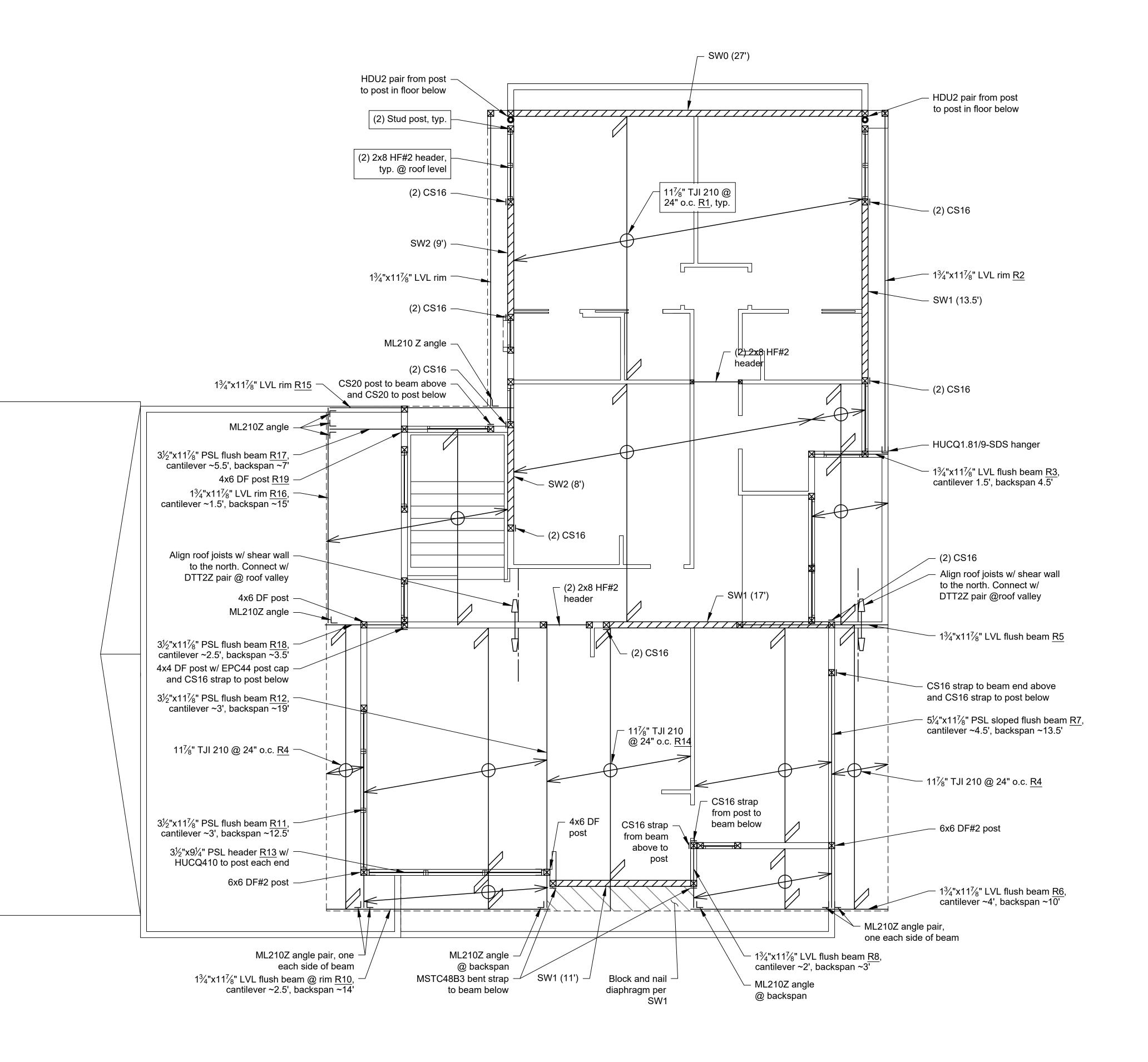


2-16-23

Scale: 1/4" = 1'-0"

S-3





Email: john@cse

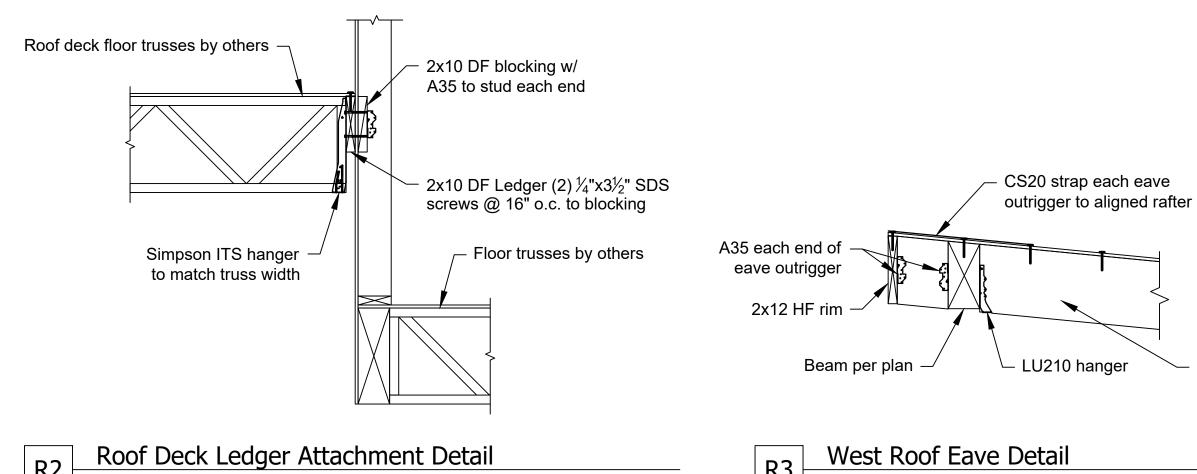
Date:

10-24-22

Sheet:

Scale: 1" = 1'-0"

S-4



(Minimum 3) #4x12"

dowels, epoxy grouted into

existing foundation. Dowels

shall be spaced minimum 6"

apart, and no less than 4"

provide dowels at 12" o.c.

vertically, for the full height

from top and bottom of

footing. For taller walls,

New reinforced concrete

foundation, refer to new

of the wall

foundation detail

5" deep holes drilled into



Scale: 1/2" = 1'-0"

Scale: 1/2" = 1'-0"

Scale: 1/2" = 1'-0"

⊬ 1'-6" MAX →

- Framing above per plan

- DTT1Z w/ 4" SDWH screw thru ledger to rim

2x12 HF#2 rim

2x12HF#2 ledger

A35 each side of outrigger "joist"

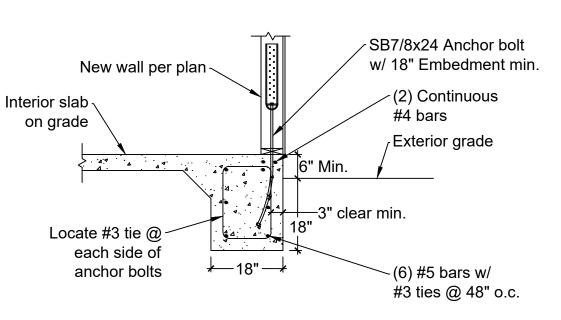
- Min. 2x12 HF#2 outriggers@ 24" o.c.

Scale: 1-1/2" = 1'-0"

New or existing wall

per structural plan

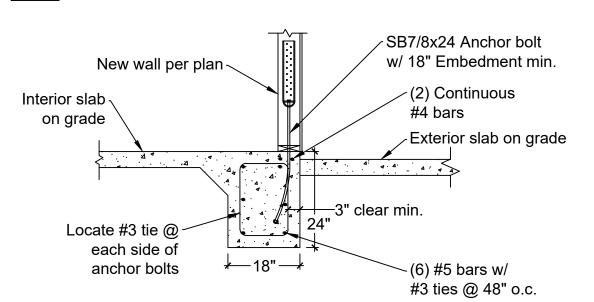
Scale: 3/4" = 1'-0"



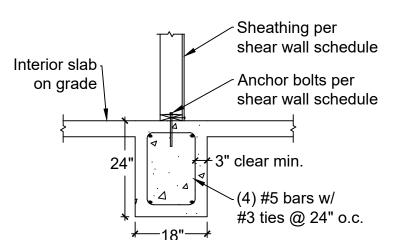


(2) $\frac{1}{4}$ "x3 $\frac{1}{2}$ " SDS screws ledger to rim –

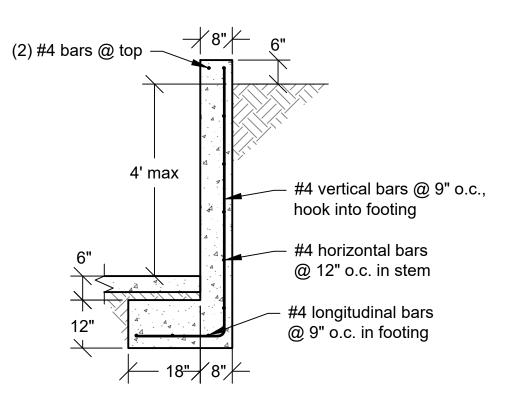
Floor framing per plan



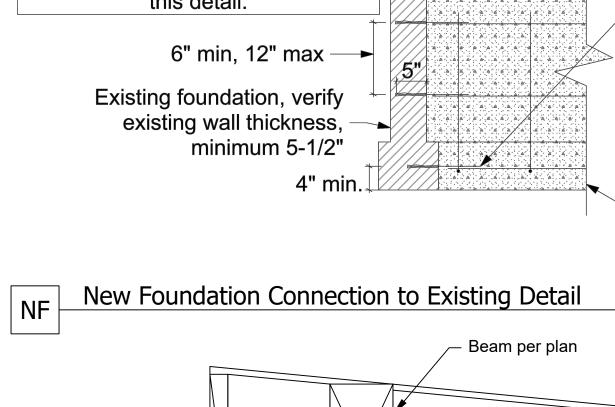
Exterior Footing Detail Scale: 1/2" = 1'-0"



Grade Beam Detail



Entry Retaining Wall Detail



Note: Contractor to verify that

existing foundation is sound and in

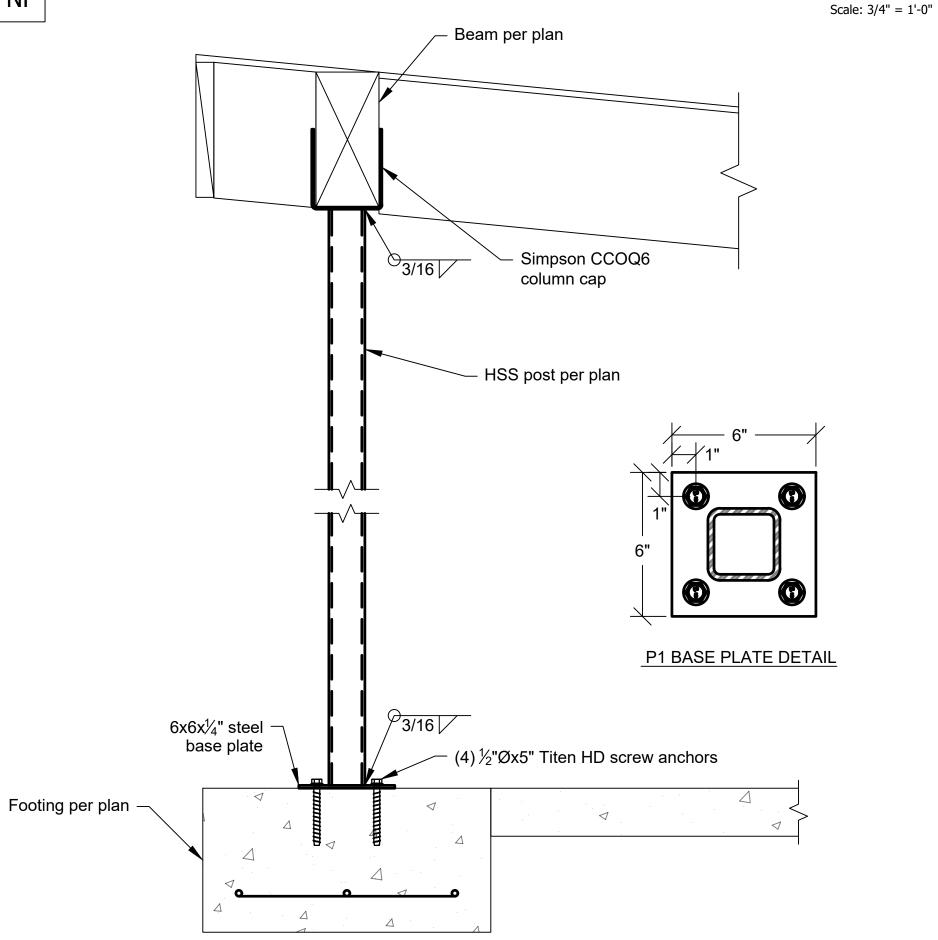
good condition. Contact the

if the existing foundation does not

meet the minimum requirements of

this detail.

engineer of record for clarfication, or



Patio Steel Post Detail

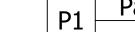
Horizontal drag strut Floor sheathing CMSTC16 strap Beam, double Alternate strap joists, or solid location, where accessible 4x_ blocking per plan Detail at floor framing Floor sheathing Horizontal drag strut CMSTC16 strap Alternate drag strut Beam, double locations, where joists, or solid accessible 4x_ blocking per plan (8) A35 angles at each strap splice between beam/joist/blocking and - Wall per plan wall top plate

Detail at wall

See note "DS" in the

structural notes for more information

Drag Strut Typical Detail



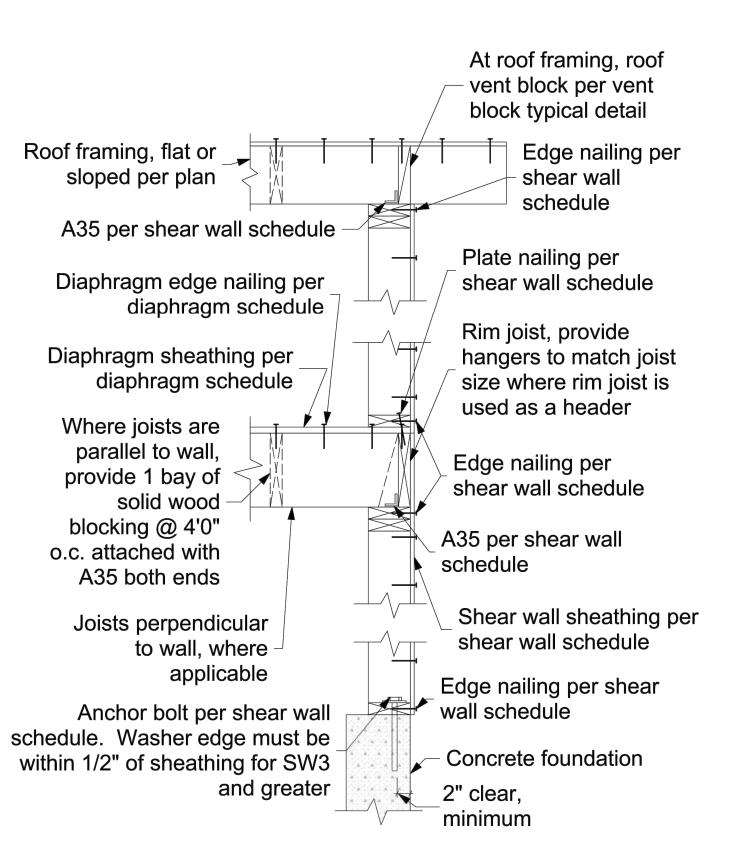
Scale: 1-1/2" = 1'-0"

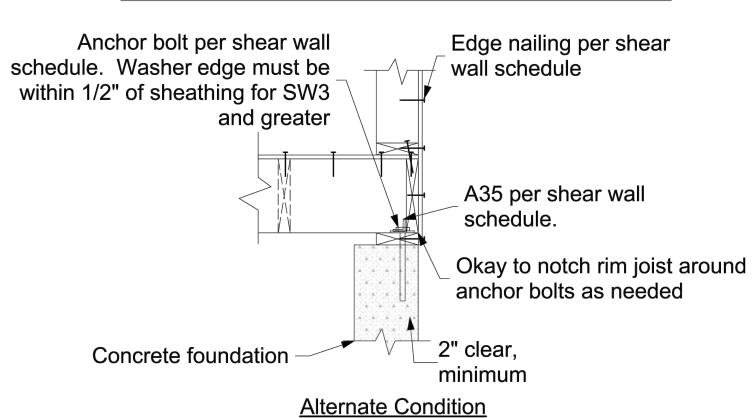
S-5

(2) 16d @ 16" o.c. min, or Shear wall above plate nailing per shear wall schedule, where applicable Diaphragm edge nailing per diaphragm schedule (where solid 2x_ blocking typical noted on the plan) · between joists, provide 4x_ Diaphragm sheathing per blocking for SW3 and greater diaphragm schedule_ Min 8d @ 12" o.c., or edge nailing per Where joists are shear wall schedule, parallel to wall, where applicable provide solid blocking @ 4'0" o.c. attached with A35 @ 4'0" o.c. min, or A35 both ends per shear wall schedule, Joists where applicable perpendicular to wall, where Shear wall sheathing per applicable shear wall & sheathing schedule

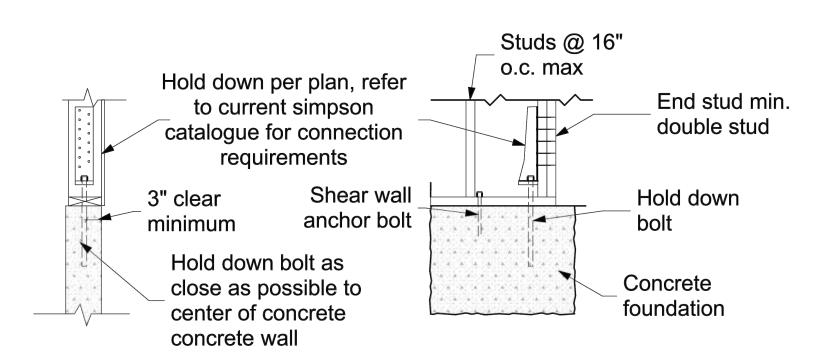
Interior Shear Wall Typical Detail

Scale: 1-1/2" = 1'-0"





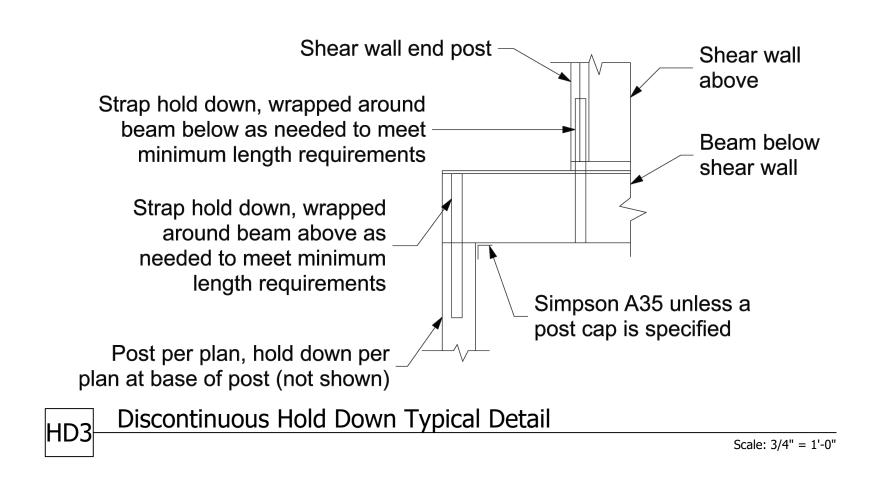
Exterior Shear Wall Typical Detail

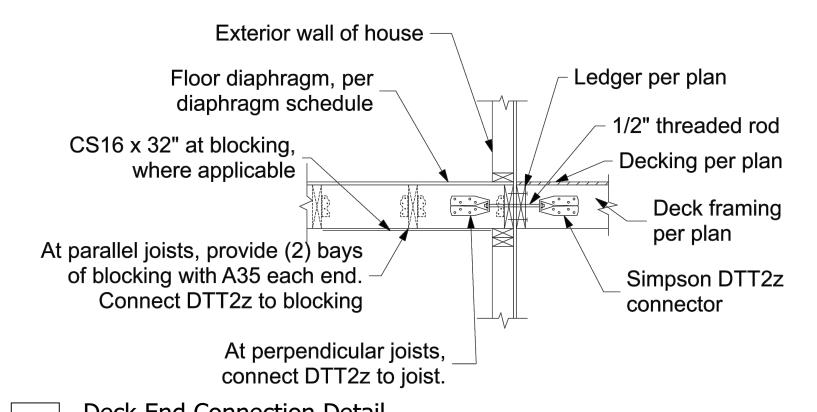


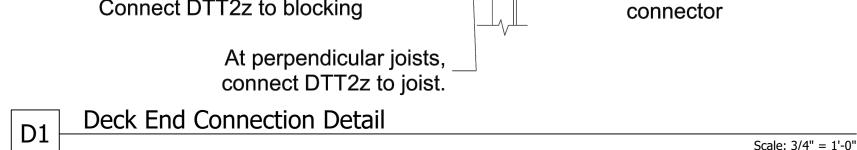
HDU Hold Down Typical Detail Scale: 3/4" = 1'-0"

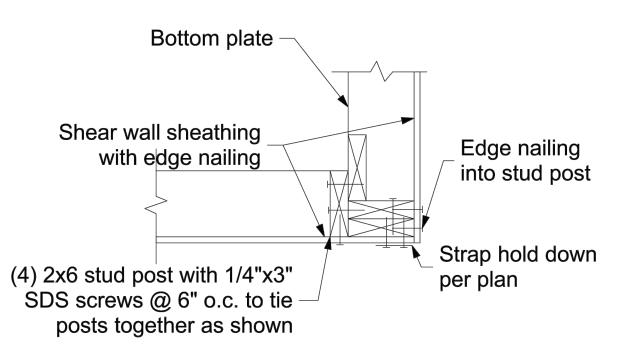
Sheathing per shear Wood framed wall per framing wall schedule Floor framing Vertical strap hold down per plan Provide vertically oriented LVL compression Strap end length, blocking for full width and - nailing per depth of post structural notes Wood post per plan [⊥]

Strap Hold Down Typical Detail Scale: 3/4" = 1'-0"

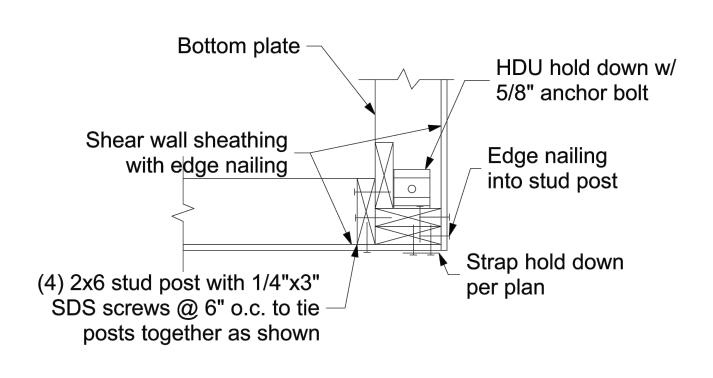








Strap Hold Down Configuration

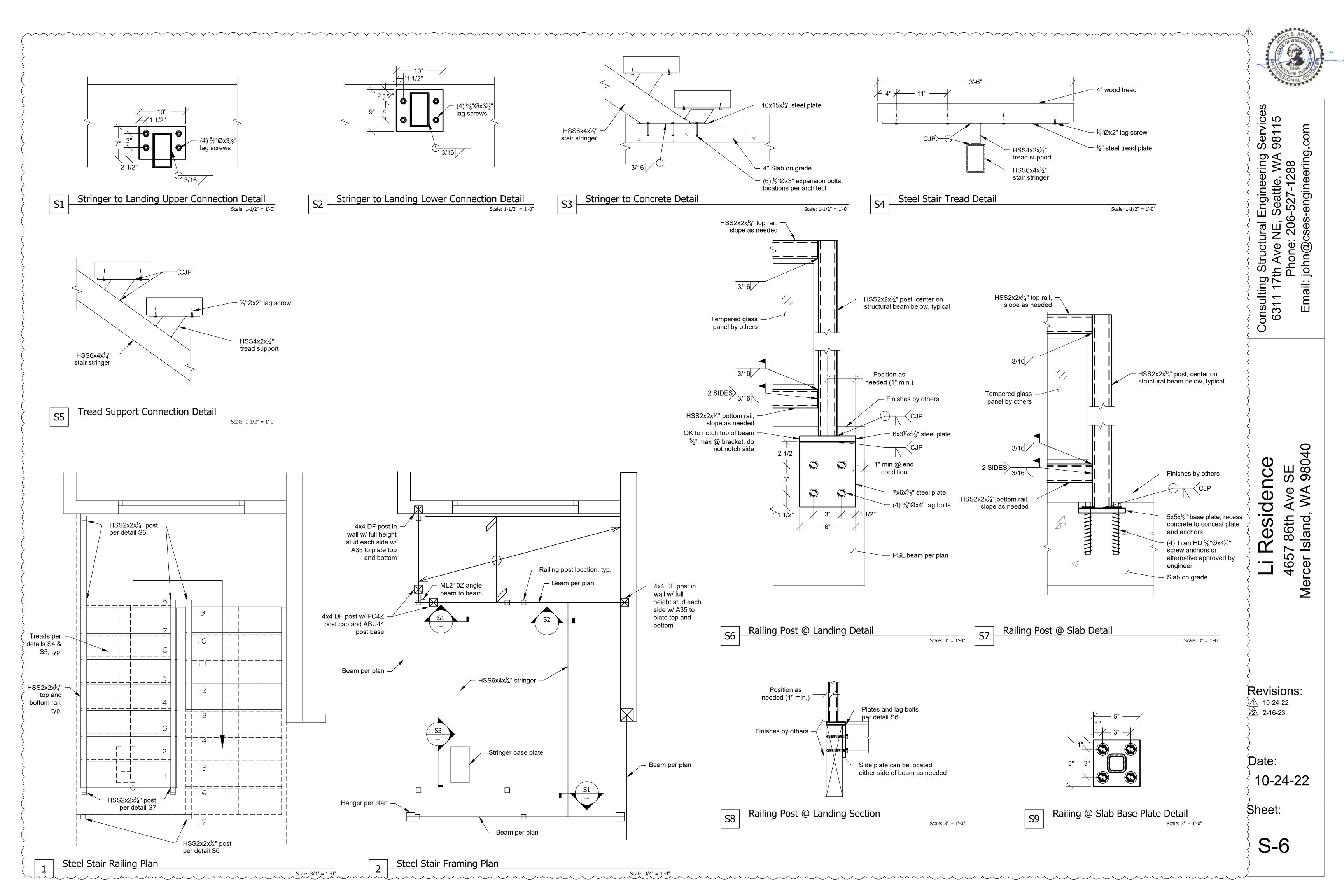


HDU Configuration

Corner Hold Down Typical Detail Scale: 1-1/2" = 1'-0"

Scale: 1" = 1'-0"

Scale: 3/4" = 1'-0"



ASCE/SEI 7-16 - "Minimum Design Loads for Buildings and Other Structures"

2018 NDS for wood structures. American Wood Preservers Bureau - AWPB Standards for Pressure Treated Material.

American Concrete Institute - ACI 315, ACI 318, ACI 301, ACI 307.

2018 International Building Code (IBC) and other applicable local building codes.

American Institute of Steel Construction - "Specification for the Design, Fabrication, and Erection of Structural Steel."

American Welding Society - AWS Structural Welding Code.

Structural design shall be in accordance with the latest edition of above codes and standards. Contractor shall comply with the latest edition of all applicable codes and standards.

Design Loads:

Live load: 25 psf (snow) 40 psf (60 psf decks) solar panels

Wind load: Basic wind speed 110 mph, exposure B, KzT=1.60

Building Category: Enclosed, Wind Important Factor Iw = 1.0 Refer to calculation page L1 for design wind forces.

Internal pressure 5 psf, Components and cladding design per 1609.6.4.4.1

Seismic loading per IBC Section 1613, Site Class D.

The basic structural type is a bearing wall system with light framed walls with shear panels. Rw = 6.5(wood structural panels), soil type D.

Seismic importance factor 1.0, Seismic Use Group I Design and Analysis by Simplified Design Procedure

Peak Ground Accelerations (PGA) based on USGS Hazards Program, by lat/long.

PGA 1 $\sec = .499$ PGA .2 $\sec = 1.436$ Seismic base shear = 0.147 * Dead Load

Foundations:

Soil parameters (assumed): Vertical allowable soil pressure: 1,500 psf

All soil conditions are to be field verified during construction. Footings shall bear on firm natural soils or on structural fill placed over firm natural soils, and inspected in place. Footings shall extend 18 inches minimum below adjacent exterior finished grade and shall extend 12 inches minimum below existing interior grade unless otherwise noted on plans. Structural fill shall be placed in 12-inch maximum horizontal lifts (loose thickness) and compacted to 90 percent of maximum dry density in accordance with ASTM D-1557. Imported structural fill shall be granular material containing no more than 5 percent fines, passing no. 200 sieve. Structural fill in place shall be tested by a licensed soil engineer or approved by the building inspector.

Drainage behind the concrete walls shall be provided conforming to the construction details.

Cast in Place Concrete:

Concrete shall attain a minimum compressive strength of 2,500 psi at 28 days (5-1/2 sack mix). An alternate mix provided by the concrete supplier and pre-approved by the building department is acceptable. Reinforcing steel shall conform to ASTM A-615, Grade 60 (Fy=60,000 psi) for all bars. Provide all wall and footing horizontal bars with 2'-0" x 2'-0" corner bars of the same size at all corners and wall intersections. Minimum lap splice 48 bar diameters.

Concrete protection for reinforcement shall be:

Concrete exposed to earth or weather Concrete cast against earth

1.5" (#5 & smaller) 2" (#6 & larger) 0.75"

Structural steel:

Plates, ASTM A36, Fy=36 ksi. Shapes, ASTM A992, Fy=50 ksi. Structural Steel Tube (HSS) per ASTM A500, Fy=46 ksi. Structural Steel Pipe per ASTM A53, Fy=35 ksi.

Bolts:

Bolts which are used in connections of steel beams to other steel beams or to the concrete supporting structure shall conform to ASTM A325. Anchor bolts shall conform to F1554. All other bolts shall conform

Minimum anchor bolt size and spacing shall be $\frac{1}{2}$ " diameter bolts @ 6' o.c. Shear wall anchor bolts per the shear wall schedule

For cast-in-place anchors, provide 7" minimum embedment into the new concrete foundation.

For retrofitted anchors, provide 5" minimum embedment into the existing concrete foundation. Epoxy grout

Provide 3"x3" square x 0.229" thick bolt washers where anchor bolts connect the sill plate to the concrete foundation.

Use E70xx electrodes for welding. All fillet welds shall be 3/16" or equal to minimum thickness of member

Welding:

being welded, whichever is greater, unless otherwise shown. All welding shall conform to the provisions of AWS and shall be performed by welders certified in accordance with AWS and WABO.

Wood Framing Specifications:

All sill plates and other wood framing which is in contact with concrete or masonry must be preservativetreated in accordance with AWPA U1 and M4 standards. For anchor bolts connecting wood sill plates to concrete or masonry, provide galvanized steel washers and nuts on top of the sill, minimum washer size 3" x 3" x 1/4" thick.

Where toenails are used for stud wall construction, a minimum of (2) toenails at top and bottom of each stud shall be provided. Toenails shall be 16d nails driven at approximately a 45 degree angle, with a minimum of 1-1/2" of the nail shank shall be embedded in both the stud and the plate. End nails driven through the plate and into the stud end grain are not permitted. Simpson A34 clips at top and bottom of each stud are permitted where correct toenailing is not provided.

Wherever joists bear on a wall or beam, either a continuous rim joist or solid wood blocking must be provided. Blocking shall be connected to the joists with A35 angles at each end. Individual blocks may be omitted to allow for ducting or other openings. Consult with the engineer of record if more than 25% of the blocking is omitted.

Where LVLs are specified with a thickness greater than 1-3/4", the beam may be built up out of multiple 1-3/4" LVL beams connected per truss-joist TJ-9000 specifier's guide.

Unless noted otherwise, the following grades and species shall be used for structural lumber:

2x joists Hem-Fir #2 2x, 3x, and 4x studs DF/L standard for plywood or WSP shear walls Hem-Fir standard for other walls

Microllam LVL lumber

LVL 1.9E, Fb = 2600 psi, Fv = 285 psi (minimums) Parallam lumber 2.0 WS, Fb = 2900 psi, Fv = 290 psi (minimums) Glu-lam lumber 24F-V4 for simple span beams, 24F-V8 for cantilever beams

All framing connections shall be per Table 2304.10.1 of the IBC, unless otherwise noted.

Preservative-Treated Wood and Fasteners:

All wood in contact with concrete or masonry shall be preservative-treated, in accordance with AWPA U1 and M4 standards.

All fasteners installed in preservative-treated wood shall be hotdipped zinc-coated galvanized with a minimum coating weight complying with ASTM A 153.

Fasteners other than nails and timber rivets are permitted to be mechanically deposited zinc-coated with coating weights complying with ASTM B 695, Class 55 minimum. Plain carbon steel fasteners in wood preservated-treated with SBX/DOT or zinc borate are not required to be galvanized.

Plywood Thickness, Grade, and Nailing:

Install plywood sheets with face grain perpendicular to framing. Stagger joints in adjacent sheets. If not otherwise noted, use nailing schedule, Table 2304.6.1 of the IBC.

Manufactured Trusses:

Manufactured trusses specified on the plans are prefabricated products manufactured by a truss manufacturer. The contractor shall submit shop drawings and stamped structural design calculations for review. The manufacturer's installation instructions shall be available on the job site at the time of inspection. Truss design and shop drawings shall include location and weight of all equipment being supported by these trusses.

The truss live loading shall be per IRC Section 301.5 and Table 301.5, especially noting footnotes b and g.

The truss design shall be per IRC Sections 502.11.1 and 802.10.2, especially indicating the truss design and manufacturing shall be per ANSI/TPI 1.

The truss temporary and permanent bracing shall be per IRC Sections 502.11.2 and 802.10.3 as well as the Truss Plate Institute's Building Component Safety Information.

Truss alterations shall not occur unless the approval of a designprofessional as indicated in IRC Sections 502.11.3 and 802.10.4.

Manufactured Joists:

"TJI" Joists specified on the plans are prefabricated products manufactured by the Weyerhaeuser Corporation. The contractor shall submit shop drawings and stamped structural design calculations for review. Joist design and shop drawings shall include location and weight of all equipment being supported by these joists. The manufacturer's installation instructions shall be available on the job site at the time of inspection. Other suppliers may be used, upon approval by the engineer of record.

Provide solid blocking between TJI joists at 8' o.c. along the span.

Blocking shall be solid engineered lumber to match the joist depth. TJI blocking is not permitted. See the TJ-9001 Installation Guide for connection and framing requirements.

Metal Framing Connectors:

Unless otherwise noted: Metal framing connectors shall be manufactured by the Simpson company, or approved equal. Unless noted otherwise, use U-series joist hangers to match joist size (e.g., U210 for 2x10 joist). Provide H1 or H2.5 hurricane ties, or other connectors with similar capacity, at every roof joist or truss, and H6 or H7 at ends of roof beams and girder trusses. Where supported by wood posts, wood beams shall be connected to the tops of the posts using Simpson AC, PCZ or EPCZ post caps, and to the bottoms of the posts bearing on wood framing using Simpson AC connectors or A35 clips. Where supported by perpendicular beams, wood beams shall be connected by HU-series face mount beam hangers. Provide Simpson AB or PB post bases to connect posts to concrete foundations. Unless otherwise specified, the maximum number of nails or screws should always be installed on any connector.

Bearing Walls:

All walls supported by continuous concrete footings shall be connected to the foundation per 2018 IRC section 403.1.6. 1/2" diameter anchor bolts shall be provided at 4' o.c., or two per wall segment, minimum. Anchor bolts shall penetrate 7" into the concrete foundation.

Drag Strut Note "DS"

Provide a continuous horizontal connection between the indicated beams, walls, and blocking, using the following method.

Connect the beams, blocking, rim joist, or top plates in the line specified, using a horizontal Simpson CMSTC16 strap or alternate strap specified on the plans. Individual members must be connected together, with the strap extending 3' onto each member. Where blocking is used, the strap must be continuous across all blocking members. The strap must be nailed using 16d sinkers, with a nailing pattern per the Simpson specifications.

The strap may be installed either on top of the plywood floor diaphragm, or connecting a beam or joist, as applicable and feasible.

Beams or joists may be connected to a wall top plate by (8) A35s.

Where no parallel members occur below the strap, provide 3-1/2" wide by 5-1/2" deep (minimum) solid wood blocking in the floor or roof framing, below the strap, for nailing. The blocking should be attached to the perpendicular joists with Simpson A34 framing anchors at both ends of each block.

Straps may be installed on top of a ridge, but not on the bottom.

Refer to the latest edition of the Simpson Catalog for required nailing and other requirements.

Refer to the Drag Strut Typical Detail provided with these plans.

Connection of New Foundation to Existing, Note "NF":

At each location where the new concrete foundation abuts the existing foundation, connect the new to the existing using minimum (3) #4 by 18" long rebar dowels, epoxy grouted into 5/8" diameter by 5" deep holes drilled into the existing foundation. Each dowel shall be no closer than 3" to any edge or corner of concrete. Minimum spacing between dowels shall be 6". For concrete wall intersections longer than 3'-0" in any direction, additional dowels shall be located at 12" o.c. for the full height or length of the new

Contact the engineer (prior to construction) for evaluation and approval of the existing foundation system, if there are any significant cracks in the existing foundation within 6 feet of the new foundation, or if there is any indication that the existing foundation is in poor condition, including visible rock pockets, non-uniform concrete, spalling, noticeable settlement of the existing footing, or other distress.

Hold Down Notes

Convention for showing shear walls and hold downs: Shear walls are shown on the framing plan for the floor above. (For example, first floor shear walls will be shown on the second floor framing plan, and the shear walls for the topmost floor will be shown on the roof framing plan.) Hold downs are located at the bottom of that shear wall, and connect the end of the shear wall to wall framing or a structural beam located in the floor below the shear wall. Contact the engineer of record for clarification if needed.

Hold downs for each floor must be continuously connected to hold downs on the floor below (or to other intermediate wood framing where so indicated), until they are finally connected to the concrete foundation.

Hold downs shall be installed so as to be as far apart as is reasonable. Hold downs may be located on either the near side or the far side of the post or double stud to which they are attached. In no case shall a hold down bolt be located farther than 6" from the end of the shear wall, except with prior written approval of the engineer. Refer to the latest edition of the Simpson Catalog for details.

Where multiple studs are called out at a hold down, nail studs together with (2) 16d nails at 8" o.c. or 1/4" x 3" Simpson SDS Screws at 12" o.c.

Where a hold down post lands on a rim joist, provide full depth vertically oriented blocking under the

See Strap Hold Down Typical Detail.

Provide a vertically oriented strap hold down consisting of one or two of the Simpson vertical strap ties listed below, connecting the end stud or post of the shear wall indicated to new or existing studs in the wall framing below, or to a wood beam supporting the shear wall, where applicable. Straps shall be installed so that the minimum end length is provided to both connected posts or studs. Where a strap is connected to a beam below, the strap shall be wrapped around the beam until the minimum end length is reached.

denotes a Simpson CS16 strap, with a minim end length of 14", and (13) 8d nails each end.

CMSTC16 denotes a Simpson CMSTC16 strap, with a minim end length of 25", and (29) 16d sinker nails each end.

CMST14 denotes a Simpson CMST14 strap, with a minim end length of 34", and (38) 10d nails each

denotes a Simpson CMST12 strap, with a minim end length of 44", and (49) 10d nails each

Rod Hold Downs:

denotes a Simpson HDU(2,4,5,8,or 11)-SDS2.5 hold down. For hold down bolts at existing concrete foundations, use the following bolts:

> For HDU2,4,5: 5/8" diameter A307 threaded steel rod may be used, which shall be epoxy grouted into a 3/4" diameter hole with a minimum embedment of 10". See Retrofit HDU Typical Detail.

For hold downs at new concrete foundations, provide the following bolts.

For HDU2,4,5: Simpson SB5/8x24 may be used, installed per the most recent edition of the Simpson Strong-Tie Literature.

Simpson SB7/8x24 may be used, installed per the most recent edition of the Simpson Strong-Tie Literature.

Where the hold down is too high off of the concrete foundation to adequately connect to the specified anchor, A 7/8" diameter threaded rod and ASTM A194-2H coupler connecting to the specified anchor may be used.

Special Note:

All holes for hold down bolts which are installed into existing foundations must be inspected during the installation of the hold down. Either the building inspector, the structural engineer of record, or the special inspection agency must perform the inspection and approve it before the bolts may be epoxy grouted into the holes. The epoxy grout used must be Simpson SET-XP unless otherwise noted by the engineer of record.

For drilled holes into existing concrete, no less than 2" must be provided between the edge of the hole and the face of concrete. The Engineer of Record or Special Inspector must witness the installation of hold down bolts, including cleaning the holes with compressed air and a wire brush before the anchor is installed. The hole shall be filled with enough epoxy that when the anchor is inserted, the epoxy rises to the top of the concrete. Care shall be taken that no air bubbles persist in the epoxy.

The contractor must verify that the existing foundation stem wall is uncracked and continuous, and is sound and in good condition, within 5 feet of any retrofitted shear wall or hold down, in any direction, except with prior written approval of the engineer. The existing concrete foundation stem wall shall be at least 6" thick and 2'-6" in height. The concrete shall be of good quality, hard and uniform, with appropriate aggregate type, size and distribution, and with no visible rock pockets or other similar

Any existing cracks located within 10' of any hold down must be completely filled with an appropriate epoxy based concrete repair product. The product to be used shall be approved in writing by the engineer prior to filling the cracks.

Contact the engineer of record prior to proceeding if any of these requirements are not met, or if the installation of the hold downs results in any visible damage to the existing foundation.



ces 5

SHEAR WALL SCHEDULE

(Lumber for shear walls is HF#2 or better, unless otherwise noted.)

		Edge		A.B.			A35	Shear
Type	Material	Nailing	Field Nailing	Size/Spacing	Plate Nailing	Plates	Spacing	Capacity
SW0	15/32" WSP one side, unblocked	8d @ 6"	8d @ 12"	1/2"Ø @ 72"	(2) 16d @ 12"	2x_	24"	100 plf
SW1	15/32" WSP one side	8d @ 6"	8d @ 12"	1/2"Ø @ 48"	(2) 16d @ 9"	2x_	24"	230 plf
SW2	15/32" WSP one side	8d @ 4"	8d @ 12"	1/2"Ø @ 32"	(2) 16d @ 6"	2x_	16"	350 plf
SW3	15/32" WSP one side	10d @ 3"	10d @ 12"	5/8"Ø @ 24"	(2) 16d @ 4"	3x_	12"	550 plf
SW3X	15/32" WSP one side	10d @ 2"	10d @ 12"	5/8"Ø @ 24"	5/8"Ø x 8" Lag @ 24"	3x_	9"	710 plf

For shear wall callouts on the Structural Framing Plans: SW x (y') denotes a shear wall type "x" with a minimum length of "y" feet. See Exterior Shear Wall Typical Detail.

• For SW3 and greater: studs, plates, and blocking where two WSP panels abut shall have a minimum 3" nominal thickness. Double 2x_ members may be used for studs if the members are connected by plate nailing. Note 10d nails at WSP panel edges.

• "WSP" refers to "Wood Structural Panel", either plywood or other wood materials.

Provide double stud minimum at both ends of all shear walls.

• Maximum spacing between nails shall not exceed 12".

• At the roof or top level of any shear wall, "A35 spacing", and all other relevant connector specifications, apply to assemblies at both the top and bottom of the shear wall. At lower levels, apply to the bottom of the wall only.

• Provide floor diaphragm edge nailing per diaphragm schedule through floor plywood into blocking, parallel joist framing, or top plates (whichever applies) of all shear walls.

Provide 3x plates, and 4x rim joists, minimum, where lag screws are specified for plate nailing.

• Where shear wall edge nails are spaced closer than 3" o.c., or spaced 3" o.c. with 10d nails, foundation sill plates and all framing members receiving edge nailing from abutting panels shall not be less than a single 3x_ member. • Provide 4x_ or double 2x_ framing where A35 angles are used on both sides of one piece of wood.

• Where a shear wall terminates above the foundation level (no shear wall below), provide minimum 4x_ blocking or double joist framing (as applicable) below the shear wall."&" Plate nailing per this schedule shall be nailed into this blocking at the bottom of the shear wall. • Shear wall nails shall be placed no closer than 3/8" from a panel edge or perpendicular face of stud.

• Shear wall nailing shall be common or galvanized box nails, unless lag screws are noted. Galvanized nails shall be hot dipped or tumbled.

 Lag screw plate connectors shall penetrate 3.5" minimum, and plates or beams receiving lag screws shall have a minimum width of 3.5". • Where hold downs are specified, the shear wall bolt shall be located within 6 inches of the end of the shear wall, unless otherwise approved by the engineer of record. Minimum end studs shall be as specified in the most recent Simpson catalog.

• Shear wall edge nailing through shear wall sheathing shall be provided into all studs attached to a hold down. •Retrofit anchor bolts shall have a minimum embedment of 5" into the concrete foundation.

• Cast in place anchor bolts shall have a minimum embedment of 7" into the concrete foundation. • For SW3 and greater, foundation anchor bolt plate washers shall extend to within 1/2" of the edge of the sheathing.

• Plate nails shall be nailed into a solid wood rim joist. • 2x_ plates may be substited for 3x_ plates if panels are nailed with edge nailing directly to the rim joist.

• Where 3x plates are used, (2) 20d common nails must be used instead of (2) 16d common nails to connect study to the bottom plate. • For SW3 and greater at existing walls, Retrofit High Strength Shear Wall Typical Detail may be used. • Where Roof ventilation is required over a shear wall, see roof ventilation detail.

Diaphragm Schedule (Lumber for diaphragm construction is HF#2 or better, unless otherwise noted.)

Type	Material	Edge Nailing	Field Nailing	Edge Blocking	Remarks
Roof	15/32" CDX 24/0	8d @ 6" o.c.	8d @ 12" o.c.	no	Minimum Standard
Floor	23/32" CDX 48/24	8d @ 6" o.c.	8d @ 12" o.c.	no	Minimum Standard

• "WSP" refers to "Wood Structural Panel", either plywood or other wood materials. • Rim joists at exterior walls shall be continuous for tension. At rim joist splice locations, provide (2) CS16 horizontal straps, minimum 24" · Where roof or floor framing is cantilevered over an exterior wall below, provide solid blocking with Diaphragm edge nailing between joists. • This is the minimum required diaphragm construction. Where otherwise noted on the plans, additional blocking or nailing may be required.

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Revisions:

10-24-22

2 2-16-23

Date:

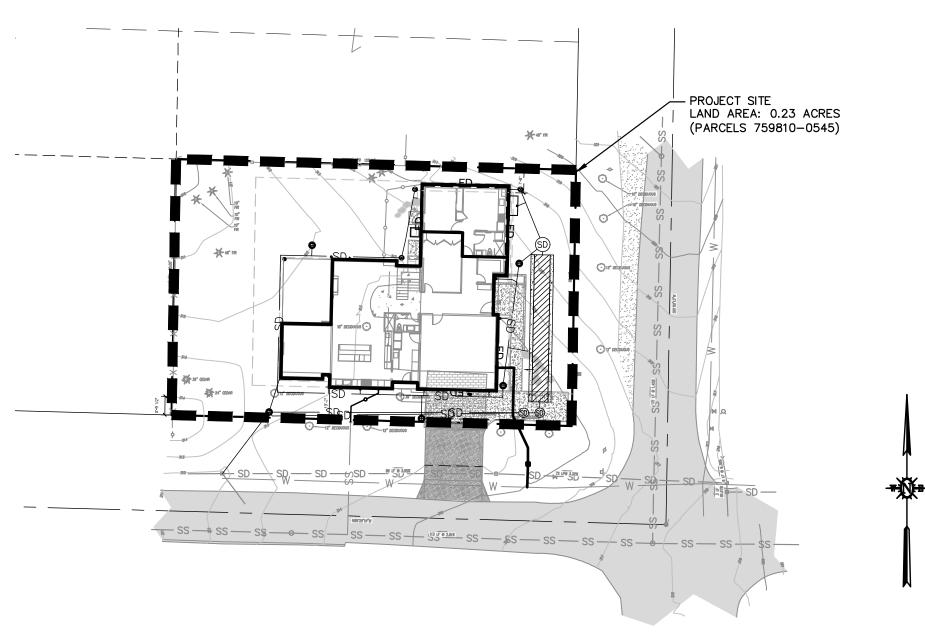
10-24-22

Sheet:

GENERAL SITE NOTES

- 1. EXISTING FEATURES, TOPOGRAPHIC AND BOUNDARY INFORMATION SHOW ON THESE PLANS ARE FROM TOPOGRAPHIC SURVEY PROVIDED BY APEX, DATED 12/17/2021
- 2. ALL WORK SHALL BE PERFORMED IN CONFORMANCE WITH THE FOLLOWING:
- A. STANDARDS OF THE UNITED STATES DEPARTMENT OF LABOR, OCCUPATIONAL SAFETY AND HEALTH ADMINISTRATION, OFFICE OF STANDARDS AND RULES OF THE STATE DIVISION OF OCCUPATIONAL SAFETY AND HEALTH,
- B. THE REQUIREMENTS OF ALL PERMITS ISSUED FOR WORK BY THE CITY OF MERCER ISLAND. WHERE CONFLICTS EXIST BETWEEN ANY OF THE ABOVE LISTED SPECIFICATIONS, THE MOST STRINGENT LISTED SPECIFICATION SHALL APPLY.
- 3. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO SECURE ALL PERMITS NECESSARY TO PERFORM WORK. INCLUDING BUT NOT LIMITED TO WORK WITHIN THE PUBLIC RIGHT-OF-WAY, GRADING, TREE REMOVAL, AND UTILITY MODIFICATIONS.
- 4. CONTRACTOR SHALL SUPPLY ALL EQUIPMENT, LABOR, AND MATERIALS NECESSARY TO PERFORM THE WORK SHOWN ON THE APPROVED PLANS.
- 5. IT SHALL BE THE RESPONSIBILITY OF THE VARIOUS CONTRACTORS TO COORDINATE THEIR WORK SO AS TO ELIMINATE CONFLICTS AND WORK TOWARD THE GENERAL GOOD AND COMPLETION OF THE ENTIRE PROJECT.
- 6. ALL WORKMANSHIP AND MATERIALS FURNISHED BY THE CONTRACTOR SHALL BE OF GOOD QUALITY AND NEW. NEITHER FINAL ACCEPTANCE NOR FINAL PAYMENT BY THE OWNER SHALL RELIEVE THE CONTRACTOR OF RESPONSIBILITY FOR FAULTY MATERIALS OR
- 7. IN THE EVENT OF ANY CONFLICT OF INFORMATION SHOWN ON THE APPROVED PLANS OR ANY CONFLICT BETWEEN THE APPROVED PLANS AND THE INTENT OF A CONSISTENT AND FUNCTIONAL PRODUCT, THE CONTRACTOR SHALL SO NOTIFY THE OWNER IN WRITING, UPON WHICH NOTICE THE OWNER SHALL RESOLVE THE CONFLICTS BY THE ISSUANCE OF A WRITTEN ORDER, REVISED PLANS, OR BOTH. THE CONTRACTOR SHALL BEAR FULL COST AND RESPONSIBILITY FOR WORK AFFECTED BY SUCH CONFLICTS AND PERFORMED BY CONTRACTOR PRIOR TO SUCH NOTICE TO THE OWNER AND ISSUANCE OF SUCH ORDER AND/OR REVISED PLANS.
- 8. CONTRACTOR SHALL EXERCISE ALL NECESSARY CAUTION TO AVOID DAMAGE TO ANY EXISTING TREES, OR SURFACE IMPROVEMENTS, OR TO ANY EXISTING DRAINAGE STRUCTURE, WATER STRUCTURE, SEWER CLEANOUTS, MANHOLES, OR JUNCTION BOXES FOR UNDERGROUND ELECTRIC, GAS, TELEPHONE, CABLE TV. STORM, SANITARY, WATER OR OTHER UTILITIES WHICH ARE TO REMAIN IN PLACE AND SHALL BEAR FULL RESPONSIBILITY FOR ANY DAMAGE THERETO.
- 9. ALL KNOWN EXISTING UTILITY LINES ARE SHOWN FOR INFORMATION ONLY, CONTRACTOR SHALL EXERCISE ALL NECESSARY CAUTION TO ANY EXISTING UTILITY LINES OR FACILITIES TO REMAIN IN PLACE. WHETHER OR NOT SUCH FACILITATES APPEAR ON THE APPROVED PLANS, AND SHALL BEAR FULL RESPONSIBILITY FOR ANY DAMAGE THERETO.
- 10. CONTRACTOR SHALL CONTACT THE "CALL BEFORE YOU DIG" UNDERGROUND UTILITY LOCATING SERVICE (811) AND THE AFFECTED UTILITY COMPANY PRIOR TO STARTING WORK TO REQUEST AND OBTAIN MARKING OF EXISTING UNDERGROUND FACILITIES.
- 11. CONTRACTORS SHALL HIRE A LICENSED LAND SURVEYOR TO PROVIDE CONSTRUCTION STAKING IN ORDER TO ENSURE THE PROJECT IS CONSTRUCTED TO THE LINES AND GRADES INDICATED ON THE APPROVED PLANS.
- 12. INSPECTION OF WORK: CONTRACTOR SHALL BE RESPONSIBLE FOR THE COORDINATION OF REQUIRE INSPECTIONS WITH THE APPROPRIATE AGENCIES AND UTILITY COMPANIES AND CITY OF MEDINA STANDARDS.
- 13. ENGINEER OF RECORD SHALL BEAR NO RESPONSIBILITY FOR METHODS AND PROCEDURES OF WORK ESTABLISHED BY CONTRACTOR. JOBSITE CONDITIONS, JOBSITE SAFETY, OR CONFORMANCE WITH SAFETY PROCEDURES AND REQUIREMENTS.
- 14. IN CONFORMANCE WITH GENERALLY ACCEPTED CONSTRUCTION PRACTICES, THE CONTRACTOR SHALL ASSUME SOLE AND COMPLETE RESPONSIBILITY FOR THE JOBSITE CONDITIONS DURING THE COURSE OF CONSTRUCTION OF THE PROJECT, INCLUDING SAFETY OF ALL PERSONS AND PROPERTY. THIS REQUIREMENT SHALL APPLY CONTINUOUSLY AND SHALL NOT BE LIMITED TO NORMAL WORKING HOURS. THE CONTRACTOR SHALL INDEMNIFY AND HOLD HARMLESS BOTH THE OWNER AND ENGINEER FROM ANY AND ALL LIABILITY REAL OR ALLEGED IN CONNECTION WITH THE PERFORMANCE OF THE WORK ON THIS PROJECT, EXCEPTING LIABILITY ARISING FROM THE SOLE NEGLIGENCE OF THE OWNER OR ENGINEER OF RECORD.
- 15. ALL UNSUITABLE MATERIALS SHALL BE REMOVED FROM THE PROJECT AND BE PLACED AT A SUITABLE DISPOSAL SITE.
- 16. AN EMERGENCY ON-SITE BACK-UP POWER SUPPLY AND AN EXTERNAL ALARM SYSTEM FOR SYSTEM FAILURE AND HIGH WATER LEVEL INDICATOR ARE REQUIRED FOR THE PUMP
- 17. PRIVATE PROPERTY OWNERS SHALL BE RESPONSIBLE FOR ANY AND ALL CLAIMS FOR INJURES AND DAMAGE DUE TO THE OPERATION OR NON-OPERATION OF THE PUMP
- 18. THE LAWN AND LANDSCAPE AREAS ARE REQUIRED TO PROVIDE POST-CONSTRUCTION SOIL QUALITY AND DEPTH IN ACCORDANCE WITH BMP T5.13. THE PROJECT CIVIL ENGINEER MUST PROVIDE A LETTER OF CERTIFICATION TO ENSURE THAT THE LAWN AND LANDSCAPE AREAS ARE MEETING THE POST-CONSTRUCTION SOIL QUALITY AND DEPTH REQUIREMENTS SPECIFIED ON THE APPROVED PLAN SET PRIOR TO FINAL INSPECTION OF THE PROJECT.
- 19. AN EMERGENCY ON-SITE, BACK-UP POWER SUPPLY AND AN EXTERNAL ALARM SYSTEM FOR SYSTEM FAILURE AND HIGH WATER LEVEL INDICATOR ARE REQUIRED FOR THE PUMP
- 20. PRIVATE PROPERTY OWNERS SHALL BE RESPONSIBLE FOR ANY AND ALL CLAIMS FOR INJURIES AND DAMAGE DUE TO THE OPERATION OR NON-OPERATION OF THE PUMP

LI RESIDENCE 4657 86TH AVE. SE MERCER ISLAND, WASHINGTON



PROJECT SITE

1" = 30'

East Seattle

PROJECT INFORMATION:

SITE ADDRESS: 4657 86TH AVE. SE, MERCER ISLAND, WA 98040

PARCEL NUMBER: 7598100545

LEGAL DESCRIPTION: (PER STATUTORY WARRANTY DEED, KING CO, REC. NO. 20211005001771)

THE SOUTH 80 FEET OF THE EAST 125 FEET OF TRACT 5 IN BLOCK 14 OF THE VITUS SCHMID'S EAST SEATTLE, ACRE TRACTS, AS PER PLAT RECORDED IN VOLUME 7 OF PLATS, PAGE 76, RECORDS OF KING

SITUATE IN THE CITY OF MERCER ISLAND, COUNTY OF KING, STATE OF WASHINGTON.

UTILITIES:

COUNTY AUDITOR;

WATER: CITY OF MERCER ISLAND 611 SE 36TH STREET, MERCER ISLAND, WA 98040 (206) 275-7602

SEWER: CITY OF MERCER ISLAND 611 SE 36TH STREET, MERCER ISLAND. WA 98040 (206) 275-7602

FIRE: MERCER ISLAND FIRE DEPARTMENT

ELECTRIC/GAS: PSE

OWNER:

PAUL LI (703)965-9722

DEVELOPER:

CAMERON WEAVER WW SUSTAINABLE, LLC 23613 140TH AVE. SE KENT, WA 98042

CIVIL ENGINEER:

ATTN: KATIE ROLLINS PE: ED COLLINS PHONE: (425) 677-2325 1700 NW GILMAN BLVD, SUITE 100 ISSAQUAH, WA 98027

SURVEYOR:

APEX ENGINEERING LLC 2601 SOUTH 35TH STREET, SUITE 200 TACOMA, WA 98409 253 473-0599

ARCHITECT:

LURE. DESIGN SOLUTIONS 13842 NE 8TH STREET, #E102 BELLEVUE, WA 98005 (425) 870-0383



- C4 GRADING AND DRAINAGE DETAILS
- C5 GRADING AND DRAINAGE DETAILS

ABBREVIATIONS

ASPHALT CC	NCRETE	AC	EDGE OF PAVEMENT	EP
CENTERLINE		CL / Q	MANHOLE RIM ELEVATION	RIM
BACK OF WA	NLK	BOW	MATCH EXISTING	ME
EACH WAY		E.W.	ON CENTER	O.C.
EXISTING GR	ADE	EG	RIGHT OF WAY	ROW
EXISTING		(E)	STANDARD	STD
FINISHED SU	REACE	FS	STORM DRAIN	SD
FINISHED GR		FG	SANITARY SEWER	SS
HIGH POINT		HP	TOP OF CURB	TC
INVERT		INV	TOP OF GRATE	TG
MINIMUM		MIN	BOTTOM OF WALL	BW
PROPERTY L	INF	PL / PL	TOP OF WALL	TW
	TY EASEMENT	PUE	UNLESS NOTED OTHERWISE	U.N.O.
			UNIFORM PLUMBING CODE	UPC

<u>LEGEND</u>		
	EXISTING	<u>PROPOSED</u>
LOT BOUNDARY PROPERTY LINE EASEMENT/SETBACK STREET CENTERLINE		
BUILDING		
GRADE BREAK CONTOURS	= = =	
WATER MAIN OR LATERAL SANITARY SEWER STORM DRAIN LINE GAS LINE	SS	——————————————————————————————————————
FOUNDATION DRAIN DRY UTILITY LATERAL		——— FD ————————————————————————————————
AC PAVING CONCRETE		
STORM DRAIN INLET		
CLEANOUT TO GRADE		©
VADD DDAIN		

YARD DRAIN

JUNCTION BOX

CATCH BASIN

DOWNSPOUT SPLASH



No. SHEET TITLE

- C1 TITLE SHEET
- C2 DRAINAGE & BMP PLAN C3 UTILITY CONEECTIONS PLAN
- C6 TESC PLAN
- C7 TESC NOTES AND STANDARD DETAILS
- C8 TESC DETAILS
- C9 TESC DETAILS



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Island



REV. NO	DATE	REVISED	DESTROY ALL PRINTS BEARING EARLIER DATE	REV. BY	CKD. BY	APRD BY	

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	Cannon
	1050 Southwood Drive San Luis Obispo, CA 93401 P 805.544.7407 F 805.544.3863
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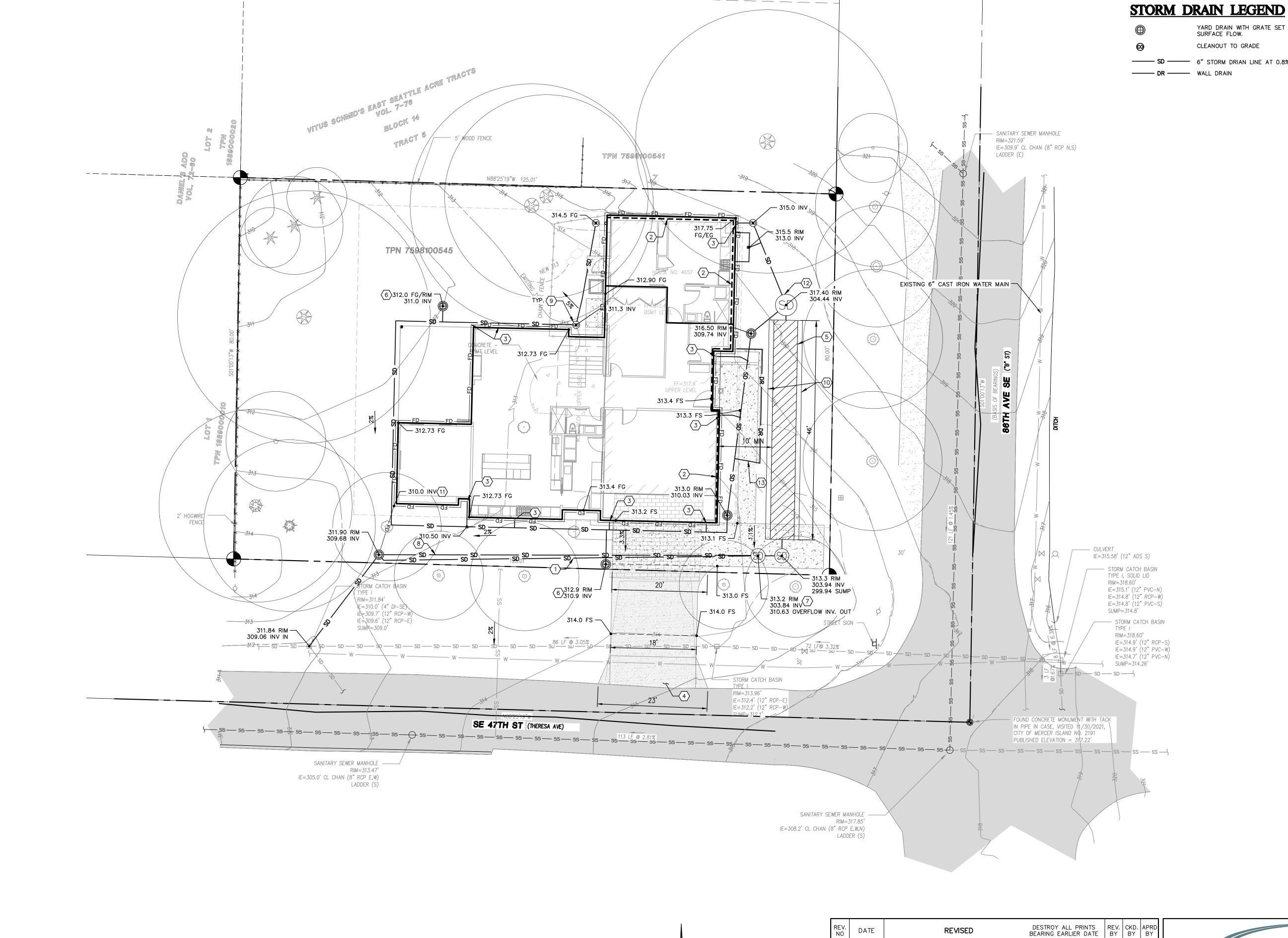
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TITLE SHEET

MERCER ISLAND, WASHINGTON

DRAWN BY	DATE	CA JOB NO.
SEM	4/12/2023	220418
CHECKED BY	SCALE	SHEET
KR	AS SHOWN	C1 OF 8





1 INCH = 10 FEET

CONSTRUCTION NOTES

(1) 6" OVERFLOW DISCHARGE PIPE.

(3) CONNECT DOWNSPOUT STORMDRAIN LINE.

- YARD DRAIN WITH GRATE SET AT ELEVATION TO ACCEPT SURFACE FLOW.
- CLEANOUT TO GRADE
- ------ SD ------ 6" STORM DRIAN LINE AT 0.8% MINIMUM SLOPE
- ASPHALT RURAL DRIVEWAY APRON PER MODIFIED KING COUNTY
- ASPHALT RURAL DRIVE WAT AFRON I LIN MODELLE STANDARD FIG 3-003 ON SHEET C4. OMIT PIPE.
- 60" DIA. DETENTION PIPE, PER DETAIL A SHEET C5.
- GEOTECHNICAL ENGINEER TO PROVIDE MEASURES TO PROTECT EXISTING
- BY CONTRACTOR AS NEEDED. (6) NYLOPLAST CATCH BASIN, PER DETAIL A SHEET C5 (GRATED COVER).

2 EXISTING RAISED FOOTING TO REMAIN. REUSE EXISTING FOUNDATION DRAIN WHERE POSSIBLE (SEE STORM DRAIN NOTE 3).

DUAL PUMP OUTLET STRUCTURE, PER DETAIL 1 SHEET C5. THE PUMP STATION STRUCTURE SHALL BE A TYPE 2 CB, WITH DIAMETER SIZED TO HOUSE THE SELECTED PUMPS AND RELATED EQUIPMENT WITH ADEQUATE ROOM FOR MAINTENANCE ACCESS (MINIMUM 48 DIA.), OR

MANUFACTURER'S PACKAGED PUMP STATION STRUCTURE SUBJECT TO

- REVIEW AND APPROVAL FROM THE PROJECT ENGINEER 8 EMERGENCY OVERFLOW PIPE.
- 9 STORM DRAIN CLEANOUT (SOLID COVER).
- CONCRETE PIPE ANCHORS PER DETAIL 2 ON SHEET C5.
 GEOTECHNICAL ENGINEER TO DETERMINE IF GROUNDWATER IS PRESENT AT BOTTOM OF EXCAVATION AND IF ANCHORS ARE
- (11) FOOTING DRAIN CONNECTION POINT TO TIGHTLINE.
- (12) UPPER CATCH BASIN PER DETAIL A SHEET C5
- (13) CONNECT BACK OF WALL DRAIN TO TIGHTLINE.

STORM DRAINAGE NOTES

1. STORM PIPE SHALL BE PVC CONFORMING TO ASTM D-3034 SDR 35 (4" - 15") OR ASTM F-679 (18"-27"). BEDDING AND BACKFILL SHALL BE AS SHÓWN IN THE STANDARD DETÁILS.

2. INSTALL FOOTING DRAINS AROUND ALL BUILDING PERIMETER FOOTINGS. THE FOOTING DRAINAGE SYSTEM AND THE ROOF DOWNSPOUT SYSTEM

- SHALL NOT BE INTERCONNECTED AND SHALL SEPARATELY CONVEY COLLECTED FLOWS TO THE CONVEYANCE SYSTEM OR TO ON-SITE STORMWATER FACILITIES. 3. EXISTING FOOTING DRAIN MUST BE TV INSPECTED. IF THE RESULT OF
- THE TV INSPECTION IS NOT IN SATISFACTORY CONDITION AS DETERMINED BY THE CITY OF MERCER ISLAND INSPECTOR, THE REPLACEMENT OF THE EXISTING FOOTING DRAIN IS REQUIRED.
- 4. CONNECT ALL ROOF DRAIN DOWNSPOUTS TO BELOW GROUND STORM DRAIN SYSTEM, UNLESS SPLASH BLOCK SHOWN.
- 5. YARD DRAIN CATCH BASINS SHALL BE NYLOPLAST DRAIN BASIN WITH 8 CIRCLE GRATE (UNLESS OTHERWISE NOTED) OR APPROVED EQUAL (DRAIN BASIN SHALL BE 12" MIN OR LARGER TO ACCOMMODATE CONNECTED PIPES). SEE DETAIL C ON SHEET C4. CATCH BASINS WITHIN DRIVEWAY OR OTHER VEHICULAR AREAS SHALL HAVE TRAFFIC RATED GRATE AND FRAME. SEE NOTE 8.
- 6. PIPES ROUTED BELOW RETAINING WALLS SHALL BE SUFFICIENTLY PROTECTED FROM WALL LOADING. FOR LANDSCAPE WALLS, PROVIDE 6" SLURRY ENCASEMENT OF PIPE AT MINIMUM WITHIN THE ZONE OF INFLUENCE OF WALL.
- NOT ON THE MAIN STORM DRAIN CONVEYANCE LINE SHALL BE COORDINATED BY LANDSCAPE ARCHITECT. 8. IF THE EXISTING CATCH BASIN IS NOT IN SATISFACTORY CONDITION, A

7. YARD DRAIN TYPE AND MANUFACTURER FOR YARD DRAINS THAT ARE

9. WHERE TWO STORM DRAINS INTERSECT AND THERE IS NO PROPOSED INLET, PROVIDE CONNECTION WITH WYE/T JUNCTION

DETERMINED BY THE CITY OF MERCER ISLAND INSPECTOR, THE REPLACEMENT OF THE EXISTING CATCH BASIN IS REQUIRED.

GENERAL NOTES

- 1. ALL GRADING AND DRAINAGE SHALL CONFORM TO THE CURRENT BUILDING CODE.
- 2. ALL DISTURBED PERVIOUS AREA SHALL BE AMENDED PER BMP T5.13 POST-CONSTRUCTION SOIL QUALITY AND DEPTH (SEE DETAIL B,
- 3. AN EMERGENCY ON-SITE, BACK-UP POWER SUPPLY AND AN EXTERNAL ALARM SYSTEM FOR SYSTEM FAILURE AND HIGH WATER LEVEL INDICATOR ARE REQUIRED FOR THE PUMP SYSTEM
- 4. PRIVATE PROPERTY OWNERS SHALL BE RESPONSIBLE FOR ANY AND ALL CLAIMS FOR INJURIES AND DAMAGE DUE TO THE OPERATION OR NON-OPERATION OF THE PUMP SYSTEM

GRADING NOTES

- 1. PRIOR TO CONSTRUCTING FLATWORK NEAR EXTERIOR DOORS, CONTRACTOR SHALL COORDINATE DOOR THRESHOLD DROP WITH ARCHITECTURAL PLANS.
- 2. ALL FLATWORK ADJACENT TO BUILDINGS SHALL BE SLOPED AT A MINIMUM OF 2% FOR 10' AWAY FROM THE BUILDING UNLESS NOTED OTHERWISE.
- 3. FINISHED GRADE (PERVIOUS AREA) DIRECTLY ADJACENT TO BUILDINGS SHALL BE SLOPED AT 5% MINIMUM FOR 10' AWAY FROM THE BUILDING, OR TO A DESIGNATED SWALE SLOPED AT 2% MINIMUM.
- 4. FINISHED GRADE (DIRT OR LANDSCAPE AREA) DIRECTLY OUTSIDE OF THE BUILDING SHALL BE 8" MINIMUM AND 12" MAXIMUM BELOW FINISHED FLOOR UNLESS NOTED WITH A SPECIALLY DESIGNED FOOTING FOOTING EMBEDMENT SHOULD MEET MINIMUM REQUIREMENTS PER STRUCTURAL ENGINEER.
- 5. NO SLOPES SHALL EXCEED 2:1 HORIZONTAL TO VERTICAL.
- 6. DRIVEWAYS SHALL NOT EXCEED 20% MAX SLOPE THROUGH EXPECTED
- 7. CONTRACTOR TO INSTALL STAIRS, STAIR LANDINGS AND HANDRAILS AS REQUIRED BY THE WASHINGTON BUILDING CODE.
- 8. PRIOR TO CONSTRUCTION, PROJECT GEOTECHNICAL ENGINEER SHALL REVIEW AND APPROVE THESE PLANS FOR CONFORMANCE WITH THEIR RECOMMENDATIONS, INCLUDING EARTHWORK AND GRADING, FOUNDATION AND WALL DRAINS, INFILTRATING STORMWATER BMPS, BUILDING PAD PREPARATION.
- 9. PROVIDE LANDINGS OUTSIDE OF ALL EXTERIOR DOORS PER WASHINGTO

LI RESIDENCE

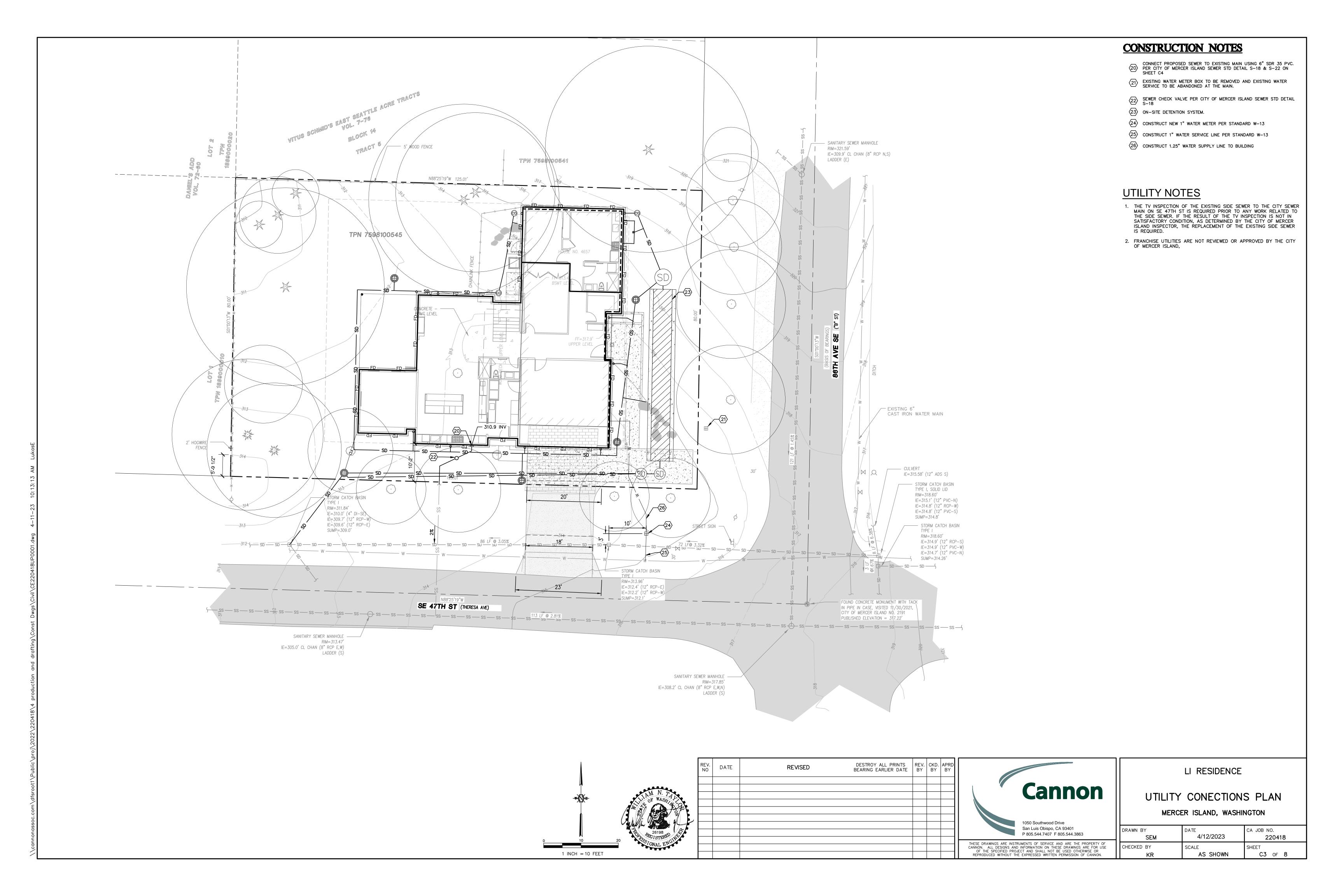
DRAINAGE & BMP PLAN

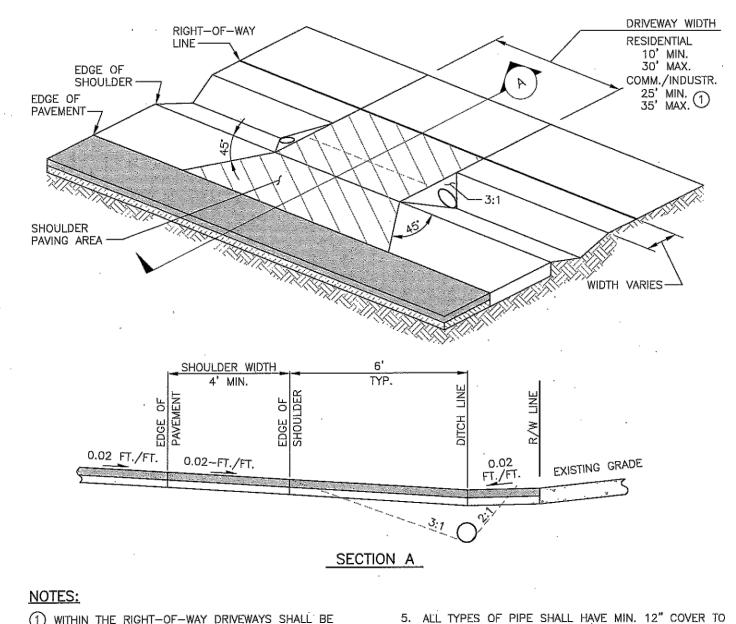
MERCER ISLAND, WASHINGTON

_	DRAWN BY SEM	DATE 4/12/2023	CA JOB NO. 220418
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	KR	AS SHOWN	C2 OF 8

Cannon 1050 Southwood Drive San Luis Obispo, CA 93401 P 805.544.7407 F 805.544.3863

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- 1) WITHIN THE RIGHT-OF-WAY DRIVEWAYS SHALL BE PAVED FROM THE RIGHT-OF-WAY LINE TO THE EDGE OF PAVEMENT WITH HOT MIX ASPHALT, NO CONCRETE IS ALLOWED WITHIN THE RIGHT-OF-WAY UNLESS AS SPECIFIED IN SEC.4.02.
- 2. COMMERCIAL/INDUSTRIAL DRIVEWAYS WIDER THAN 35 FT. MAY BE APPROVED BY THE COUNTY ROAD ENGINEER CONSIDERING BOTH TRAFFIC SAFETY AND THE ACTIVITY ALL COMMERCIAL/INDUSTRIAL DRIVEWAYS SHALL HAVE AN
- EXPANSION JOINT LOCATED MID-WIDTH, (SEE SEC. 3.04.)
- A. SIZED TO CONVEY COMPUTED STORM WATER RUNOFF, B. MIN. 12" DIAM., AND
- C. EQUAL TO OR LARGER THAN EXISTING PIPES WITHIN 500 FT. UPSTREAM. 4. EXPOSED PIPE ENDS SHALL BE BEVELED TO MATCH THE SLOPE FACE AND PROJECT NO MORE THAN 2"

BEYOND SLOPE SURFACE. PROJECTING HEADWALLS

ARE NOT ACCEPTABLE.

2016 Design and

- FINISH GRADE. 6. PIPE SHALL BE INSTALLED IN A STRAIGHT UNIFORM ALIGNMENT AT A MIN. 0.5% SLOPE (0.5 FT. PER 100 FT.) WITH THE DOWNSTREAM END LOWER THAN THE UPSTREAM END,
- PIPE MAY BE OMITTED IF ROADSIDE DITCH DOES NOT EXIST AND DRIVEWAY DOES NOT BLOCK
- NATURAL FLOW. DRIVEWAY SLOPE SHALL MATCH TO BACK EDGE OF SHOULDER, BUT SHOULDER SLOPE AND EDGE OF SHOULDER SHALL NOT BE ALTERED AS A RESULT OF

FIG. 3-003

- DRIVEWAY CONSTRUCTION. 9. SEE SEC. 3.01 AND 4.01 FOR DRIVEWAY AND SURFACING STANDARDS.
- 10. PIPING OF DITCHES SHALL BE ALLOWED ONLY WHERE DRIVEWAY ACCESS IS NECESSARY.

SHOULDER AND DITCH SECTION DRIVEWAY

DISCONNECTION

WHEN DEMOLISHING AN EXISTING BUILDING, THE BUILDING SIDE SEWER SHALL BE DISCONNECTED PRIOR TO REMOVAL OF BUILDING FOUNDATIONS. THE CONTRACTOR SHALL INSTALL A MECHANICAL PLUG WITH NON-SHRINK GROUT AT THE END OF THE SIDE SEWER TO REMAIN IN PLACE. DISCONNECTION'S SHALL BE PERFORMED IN THE PRESENCE OF THE CITY'S UTILITY INSPECTOR. THE CONTRACTOR SHALL PROVIDE AN AS-BUILT DRAWING DEPICTING THE DISCONNECTED SIDE SEWER UPON COMPLETION OF THE

RECONNECTION

WHEN RECONNECTING TO AN EXISTING SIDE SEWER, THE POINT OF RECONNECTION WILL BE DETERMINED BASED ON THE MAGNITUDE OF THE CONSTRUCTION ON THE PROPERTY.

- 1. PARTIAL INTERIOR REMODEL AND/OR BUILDING ADDITION WITH NO ADDITIONAL PLUMBING FIXTURES - NO SIDE SEWER REPLACEMENT REQUIRED UNLESS A KNOWN PROBLEM EXISTS IN THE SIDE SEWER.
- 2. PARTIAL INTERIOR REMODEL AND/OR BUILDING ADDITION WITH ADDITIONAL PLUMBING FIXTURES-ASSESS CONDITION OF EXISTING SIDE SEWER THROUGH VIDEO INSPECTION FROM BUILDING TO PROPERTY LINE AND REPLACE AS NEEDED.
- 3. COMPLETE INTERIOR REMODEL OF RESIDENCE ASSESS CONDITION OF EXISTING SIDE SEWER THROUGH VIDEO INSPECTION FROM BUILDING TO PROPERTY LINE AND REPLACE AS NEEDED. IF EXISTING SIDE SEWER IS ASBESTOS CEMENT OR CONCRETE, SIDE SEWER SHALL BE REPLACED FROM BUILDING TO PROPERTY LINE, UNLESS THE APPLICANT PROVES, TO THE SATISFACTION OF THE CITY ENGINEER, THAT THE SIDE SEWER IS WATER TIGHT AND IN SOUND CONDITION.*
- 4. COMPLETE INTERIOR REMODEL AND BUILDING ADDITION NEW SIDE SEWER FROM BUILDING TO PROPERTY LINE.*
- 5. CONSTRUCTION OF A NEW SINGLE FAMILY RESIDENCE NEW SIDE SEWER FROM BUILDING TO

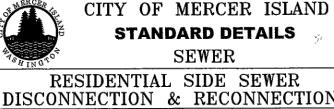
BACK WATER VALVE INSTALLATION PER CITY ENGINEER, IF SCENARIO 2, 3, 4, OR 5 IS DIRECTLY ATTACHED TO THE LAKE LINE OR THE ELEVATION OF THE LOWEST DRAIN IN THE RESIDENCE IS LOWER THAN THE RIM ELEVATION OF THE UPSTREAM SEWER MANHOLE ON THE MAIN.

VIDEO INSPECTION OF THE EXISTING SIDE SEWER, BETWEEN THE PROPERTY LINE AND THE SEWER MAIN SHALL BE PERFORMED FOR SCENARIOS NUMBER 4 AND 5.

PROVIDE A COPY OF THE VIDEO DOCUMENTATION (VIDEO AND HARDCOPY REPORT) TO THE CITY

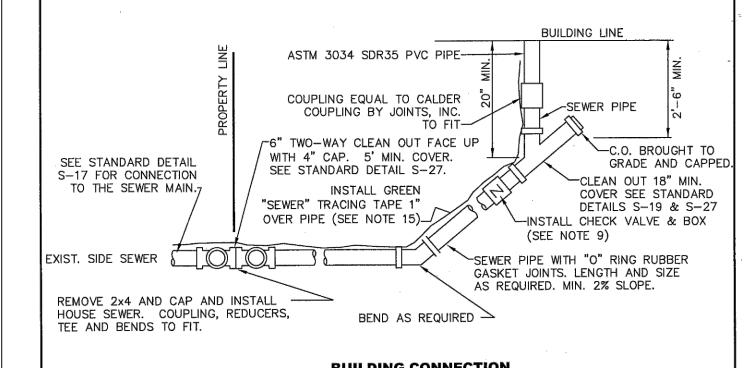
REPLACEMENT OR REPAIR OF THAT PORTION OF THE SIDE SEWER BETWEEN THE PROPERTY LINE AND THE SEWER MAIN, WILL BE DETERMINED BY THE CITY ENGINEER, BASED ON THE VIDEO INSPECTION. *IF THE EXISTING SIDE SEWER IS PVC AND IS LESS THAN TEN YEARS OLD, THE SIDE SEWER DOES NOT HAVE TO BE REPLACED IF A VIDEO INSPECTION AND/OR HYDROSTATIC PRESSURE TEST CONFIRMS THAT THE SIDE SEWER IS IN PROPER WORKING CONDITION. THESE TESTS SHALL BE PERFORMED AFTER ALL

HEAVY EQUIPMENT THAT COULD DAMAGE THE SIDE SEWER IS OFF OF THE SITE.



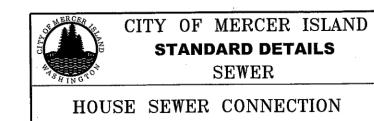
RESIDENTIAL SIDE SEWER DISCONNECTION & RECONNECTION S-22 6-5-2009 NO SCALE APPROVED

REV DATE

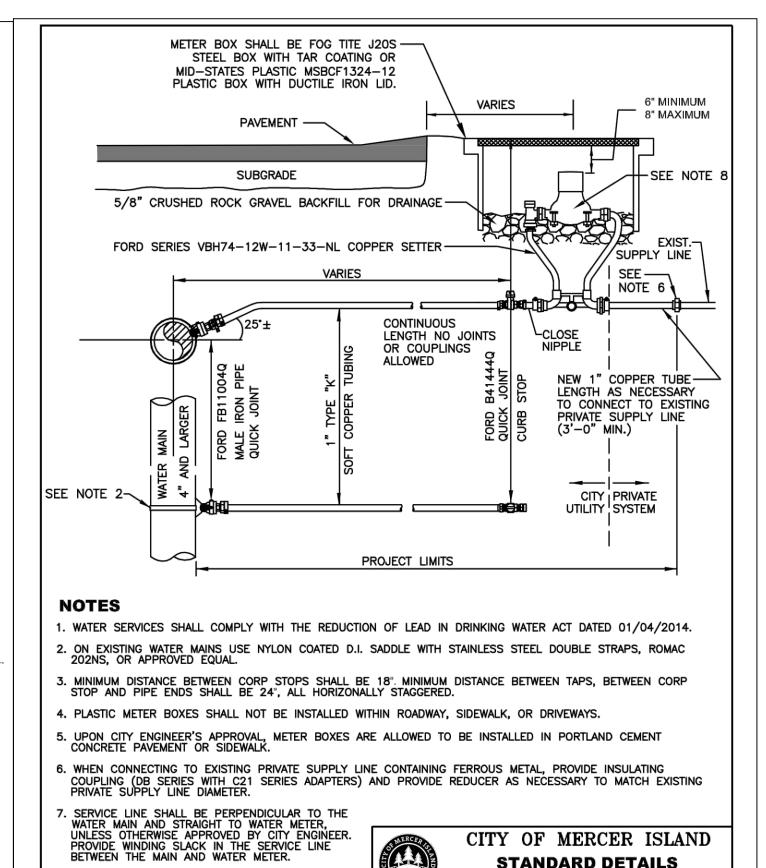


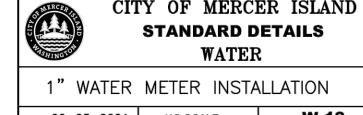
BUILDING CONNECTION

- ELBOWS SHALL NOT BE GREATER THAN 45 DEGREES. 2. CLEAN OUT IS REQUIRED FOR EACH PIPE LENGTH GREATER THAN 100' AND FOR EACH 90°
- ACCUMULATED ELBOW/100°. 3. ALL HOUSE PLUMBING OUTLETS MUST BE CONNECTED TO THE SEWER. NO DOWN SPOUTS OR STORM
- DRAINAGE MAY BE CONNECTED TO THE SEWER SYSTEM. 4. 18" MINIMUM COVERAGE OVER PIPE.
- 5. LAY PIPE IN STRAIGHT LINE BETWEEN BENDS. MAKE ALL CHANGES IN GRADE OR LINE WITH & BEND OR
- WYE. 90° CHANGE WITH 1/8 BEND AND WYE. 6. 4" SEWER PIPE MINIMUM SIZE ON PROPERTY. 2% MINIMUM GRADE.
- 7. ALL CONSTRUCTION SHALL BE IN ACCORDANCE WITH CURRENT SEWER ORDINANCES. 8. ALL CONSTRUCTION REQUIRES A PLAN SHOWING PROPERTY AND DIMENSIONS AND COMPLETION OF SIDE SEWER APPLICATION AND MAINTENANCE AGREEMENT, AS NEEDED.
- 9. BACK WATER VALVE (CHECK VALVE) IS REQUIRED: A. IF CONNECTED TO A SHARED SIDE SEWER. B, IF CONNECTION AT HOUSE IS LOWER THAN BOTH UPSTREAM AND DOWNSTREAM MANHOLE.
- C. SEE S-23 & S-24 FOR LAKE LINE REQUIREMENTS. 10. AS-BUILT DRAWING SHOWING LOCATION OF SIDE SEWER & ALL BENDS, C.O. ETC., IN RELATION TO THE HOUSE IS REQUIRED AFTER INSPECTION & INSTALLATION. SEE STANDARD DETAIL S-38 FOR A
- TYPICAL "AS BUILT" 11. THE MINIMUM PIPE SIZE FOR SIDE SEWERS SHALL BE:
 - 6" WITHIN THE PUBLIC RIGHT-OF-WAY. 4" - SINGLE FAMILY RESIDENCES.
 - 6" 2 TO 6 SINGLE FAMILY RESIDENCES. 6" - BUILDINGS OTHER THAN SINGLE FAMILY RESIDENCES
- 12. UTILITY PIPE TRACER TAPE SHALL BE DETECTABLE BELOW GROUND SURFACE, COLOR CODED, WITH UTILITY NAME PRINTED ON TAPE. CONDUCTIVE WARNING TAPE REQUIRED OVER ALL WATER PIPE. TAPE SHALL BE MANUFACTURER'S STANDARD PERMANENT, BRIGHT-COLORED, CONTINUOUS PRINTED PLASTIC TAPE, ALUMINUM BACKED, INTENDED FOR DIRECT-BURIAL SERVICE. TAPE SHALL BE NOT LESS THAN 6" WIDE X 4 MILS THICK.

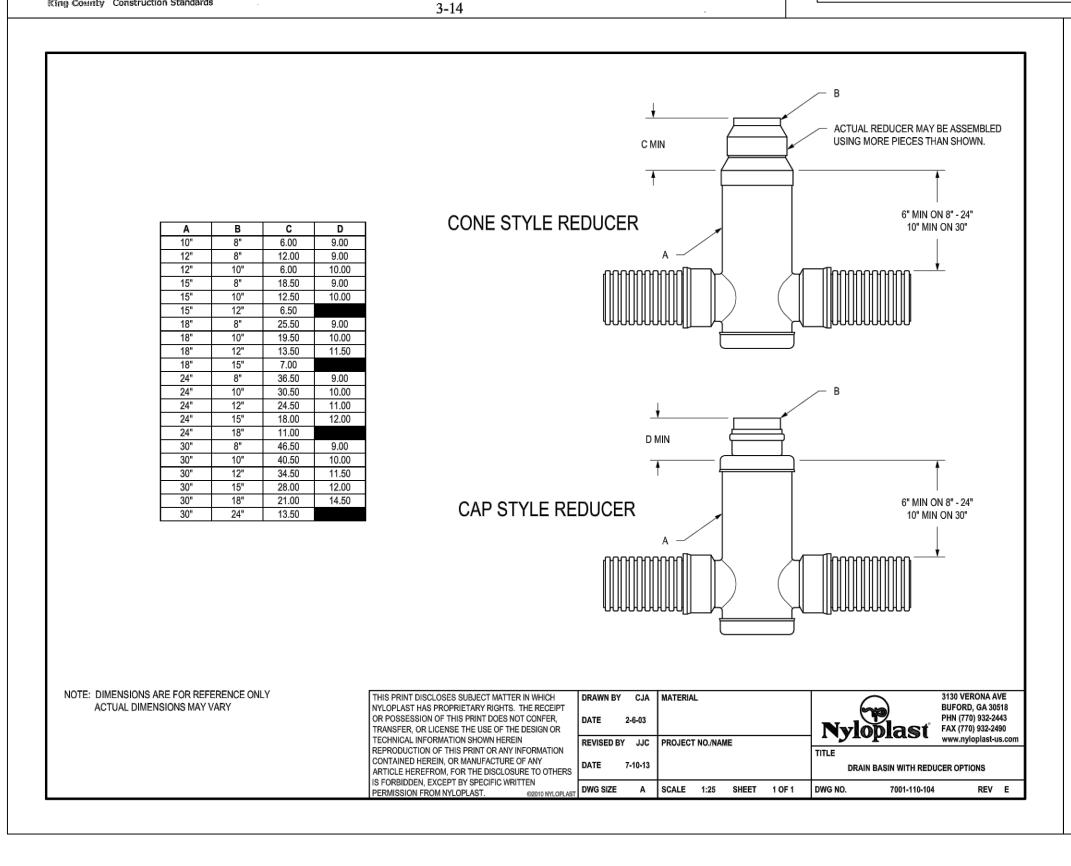


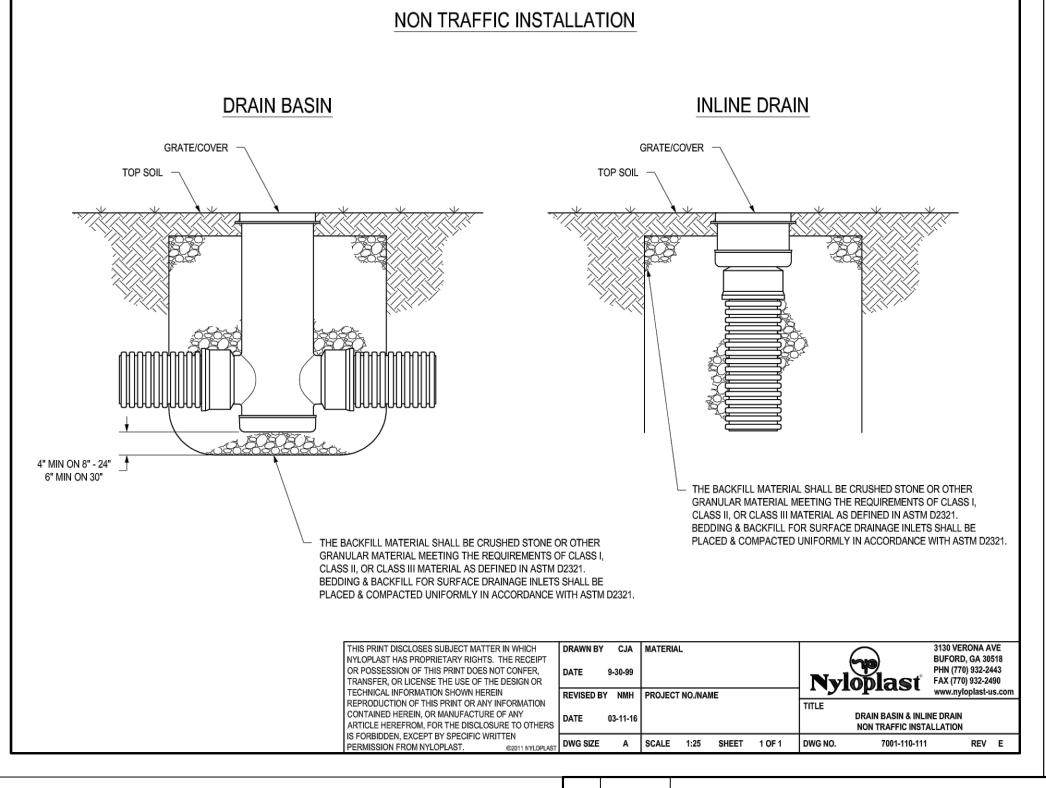
S-18 6-5-2009 NO SCALE APPROVED REV DATE

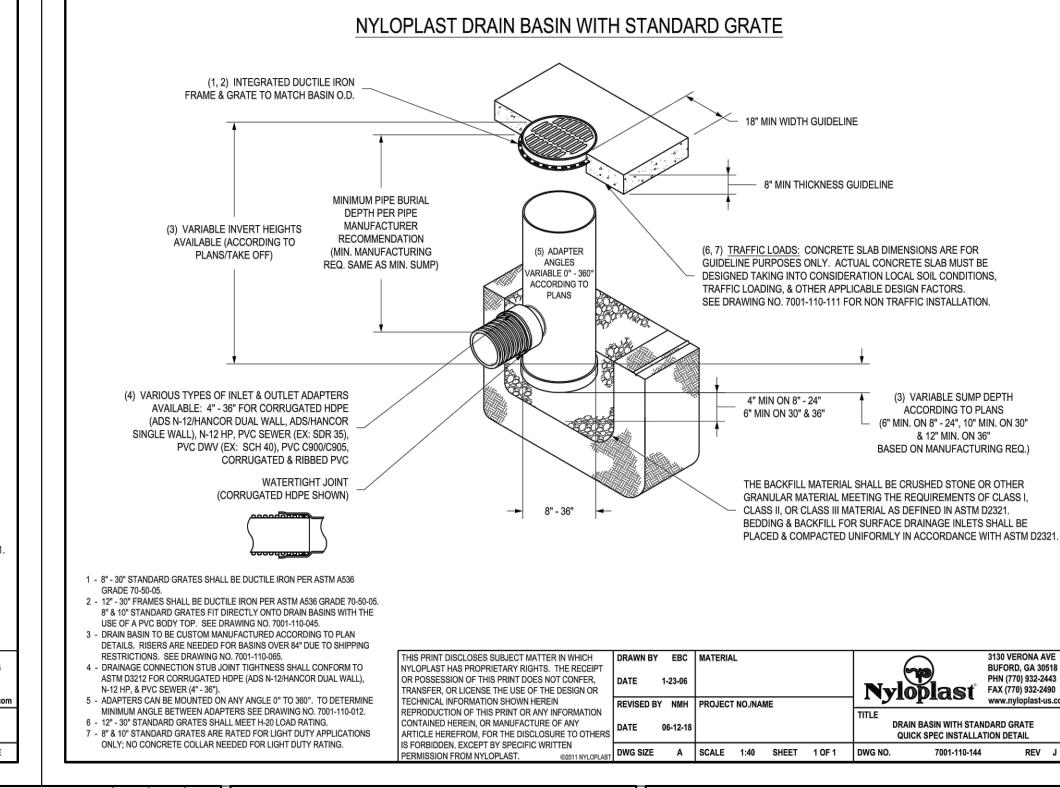




 NO SERVICE CONNECTIONS BETWEEN BLOW-OFF AND END OF MAIN. 02-05-2021 W-13 REV DATE APPROVED

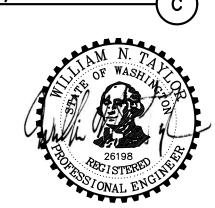






8. WATER METER SUPPLIED BY CITY.

ALL FITTINGS TO BE BRASS COMPRESSION TYPE, FORD QUICK JOINT OR EQUAL.



TYPICAL YARD DRAIN/CATCH BASIN

DESTROY ALL PRINTS | REV. | CKD. | APRI REVISED BEARING EARLIER DATE | BY | BY | BY THESE DRAWINGS ARE INSTRUMENTS OF SERVICE AND ARE THE PROPERTY OF

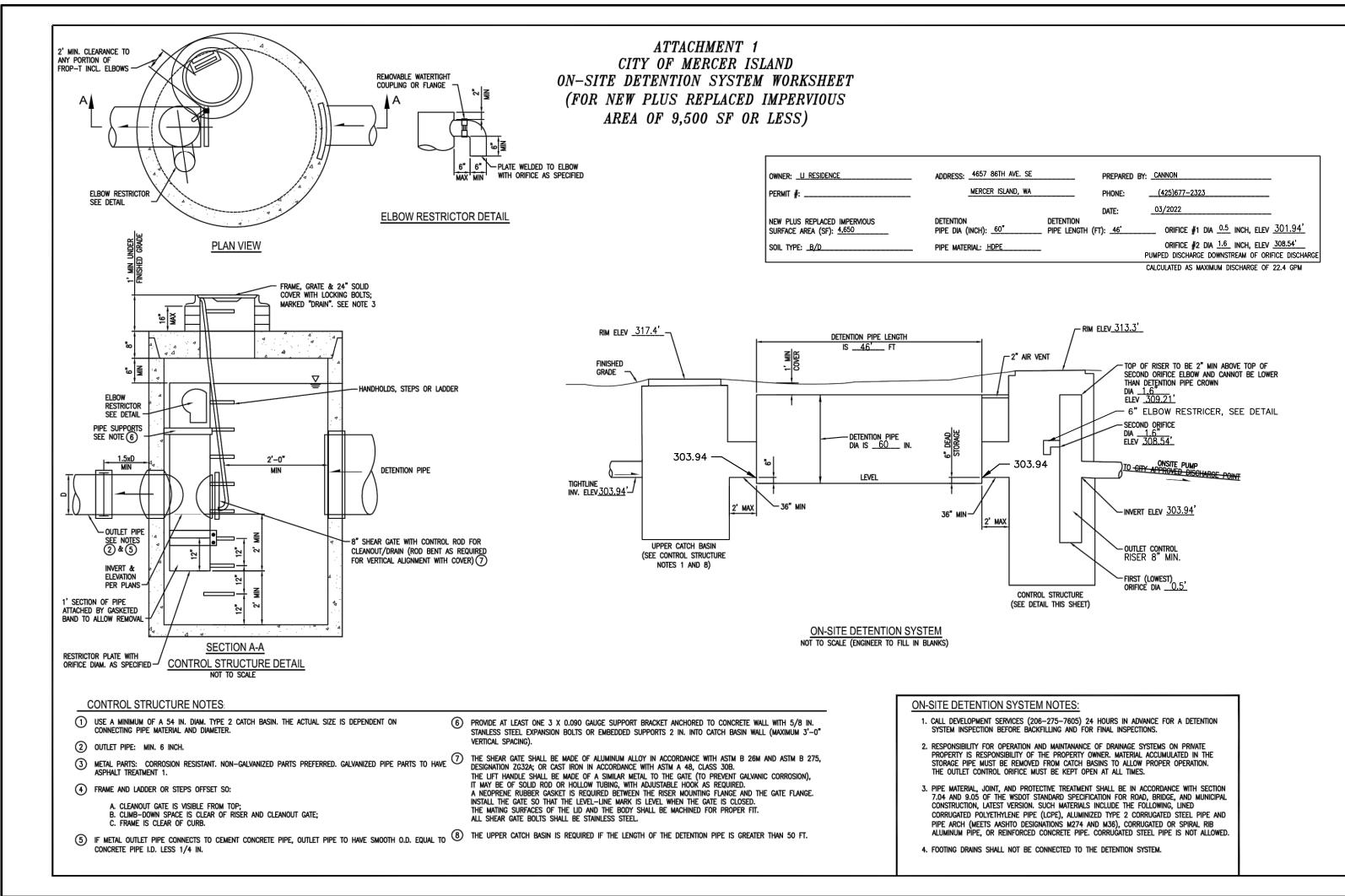


ANNON. ALL DESIGNS AND INFORMATION ON THESE DRAWINGS ARE FOR USE OF THE SPECIFIED PROJECT AND SHALL NOT BE USED OTHERWISE OR REPRODUCED WITHOUT THE EXPRESSED WRITTEN PERMISSION OF CANNON.

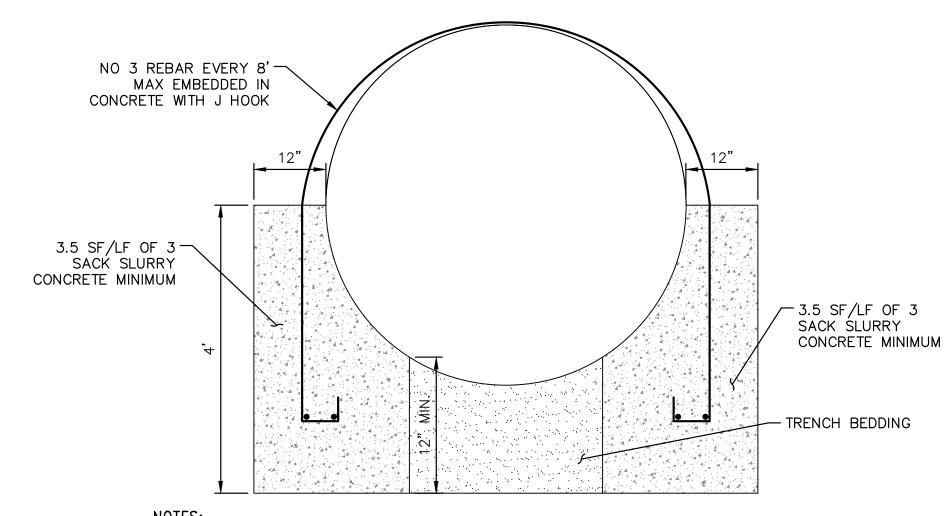
LI RESIDENCE

GRADING AND DRAINAGE DETAILS MERCER ISLAND, WASHINGTON

DRAWN BY CA JOB NO. 4/12/2023 220418 CHECKED BY SHEET AS SHOWN C4 OF 8







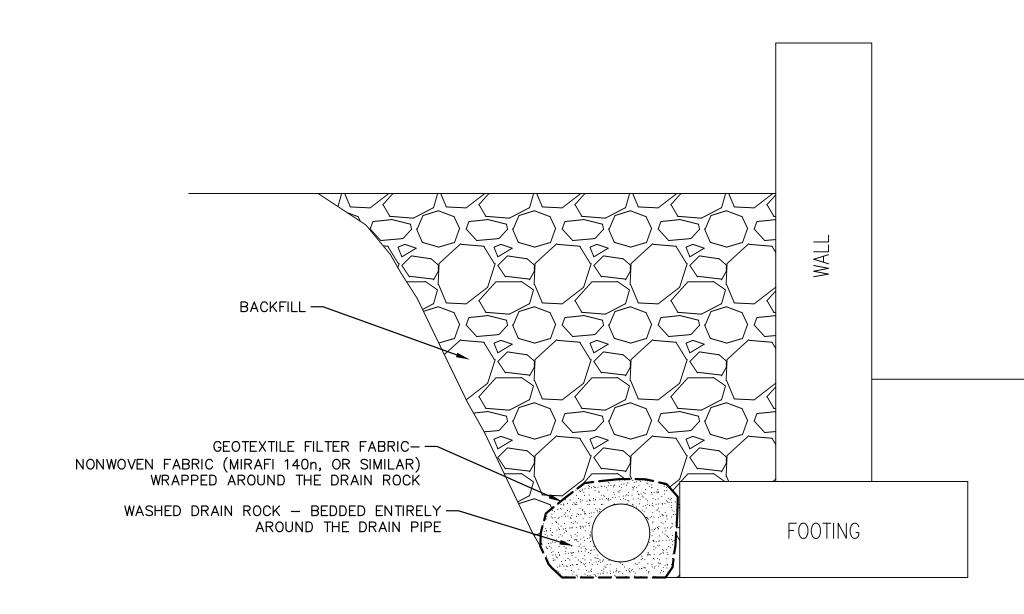
1. INSTALL PIPE ANCHOR FOR LENTH OF 60" PIPE (46 LF)

2. INSTALL PIPE ANCHOR ONLY AS REQUIRED BY GEOTECHNICAL ENGINEER IF GROUNDWATER IS PRESENT. GEOTECHNICAL ENGINEER TO REVIEW THIS DETAIL FOR CONFORMANCE WITH THEIR REQUIREMENTS.

CONCRETE PIPE ANCHOR

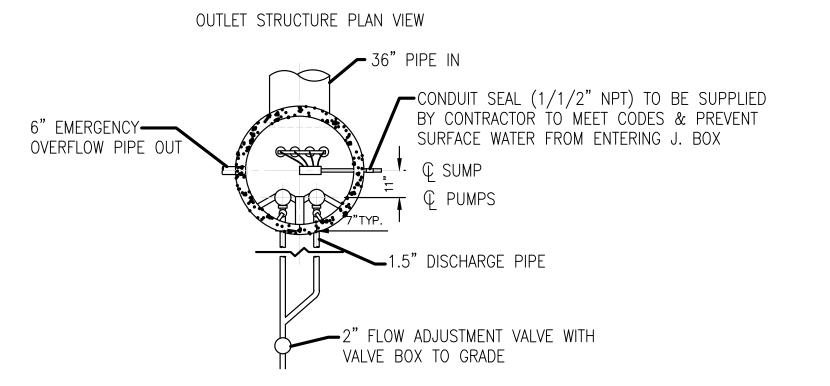
NOT TO SCALE

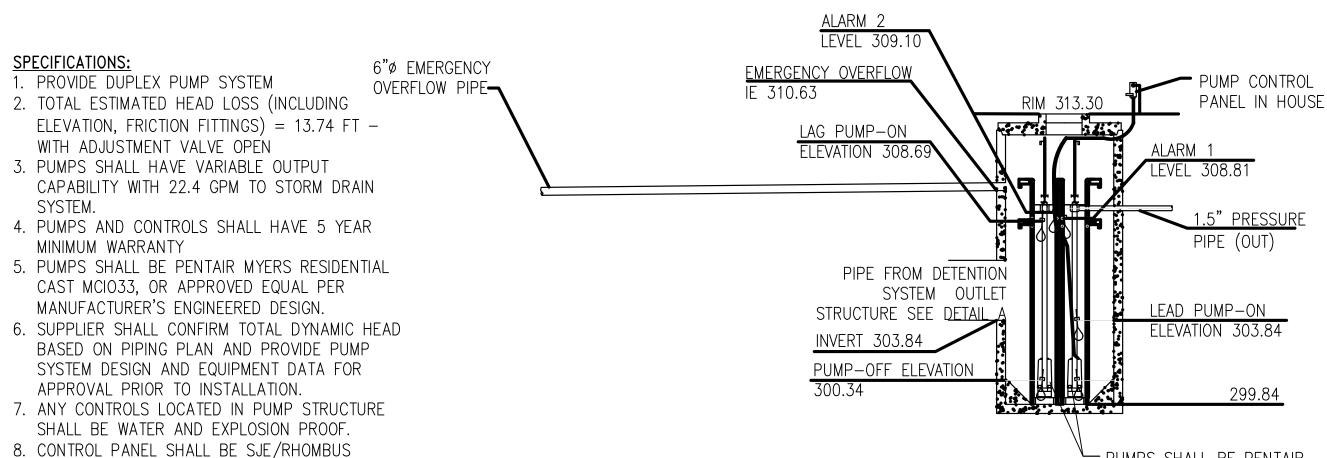




TYPICAL FOOTING DRAIN / NOT TO SCALE

- 1. DO NOT REPLACE RIGID PVC PIPE WITH FLEXIBLE CORRUGATED PIPE
- 2. PERFORATED PVC PIPE SHOULD BE TIGHT JOINTED AND LAID WITH PERFORATIONS ORIENTED DOWNWARD. THE PIPE SHOULD BE GENTLY SLOPED TO PROVIDE FLOW TOWARD THE TIGHTLINE OR DISCHARGE LOCATION.
- 3. DO NOT CONNECT OTHER DRAINAGE LINES INTO THE WALL DRAIN SYSTEM
- 4. BACKFILL SHOULD MEET STRUCTURAL FILL SPECIFICATIONS IF IT WILL SUPPORT DRIVEWAYS. SIDEWALKS, PATIOS, OR OTHER STRUCTURES. REFER TO THE GEOTECHNICAL ENGINEERING REPORT FOR STRUCTURAL FILL RECOMMENDATIONS.





CONTRACTOR, RUN LIGHTS, H-O-A SWITCH (OR

NOT TO SCALE

APPROVED EQUAL). 9. POWER SUPPLY FROM HOUSE PANEL BY CONTRACTOR SUBJECT TO BUILDING ELECTRICAL INSPECTION. COORDINATE PUMP SELECTION WITH POWER SUPPLY.

DUPLEX ALTERNATING WITH NEMA 4X

DATE

ENCLOSURE, HIGH WATER ALARM, MOTOR

STORMWATER DETENTION AND PUMP STATION

DESTROY ALL PRINTS REV. CKD. APROBEARING EARLIER DATE BY BY BY REVISED Cannon 1050 Southwood Drive San Luis Obispo, CA 93401 P 805.544.7407 F 805.544.3863 THESE DRAWINGS ARE INSTRUMENTS OF SERVICE AND ARE THE PROPERTY OF CANNON. ALL DESIGNS AND INFORMATION ON THESE DRAWINGS ARE FOR USE
OF THE SPECIFIED PROJECT AND SHALL NOT BE USED OTHERWISE OR
REPRODUCED WITHOUT THE EXPRESSED WRITTEN PERMISSION OF CANNON.

LI RESIDENCE

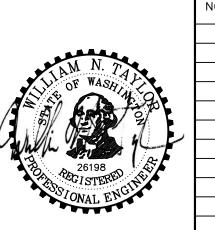
— PUMPS SHALL BE PENTAIR

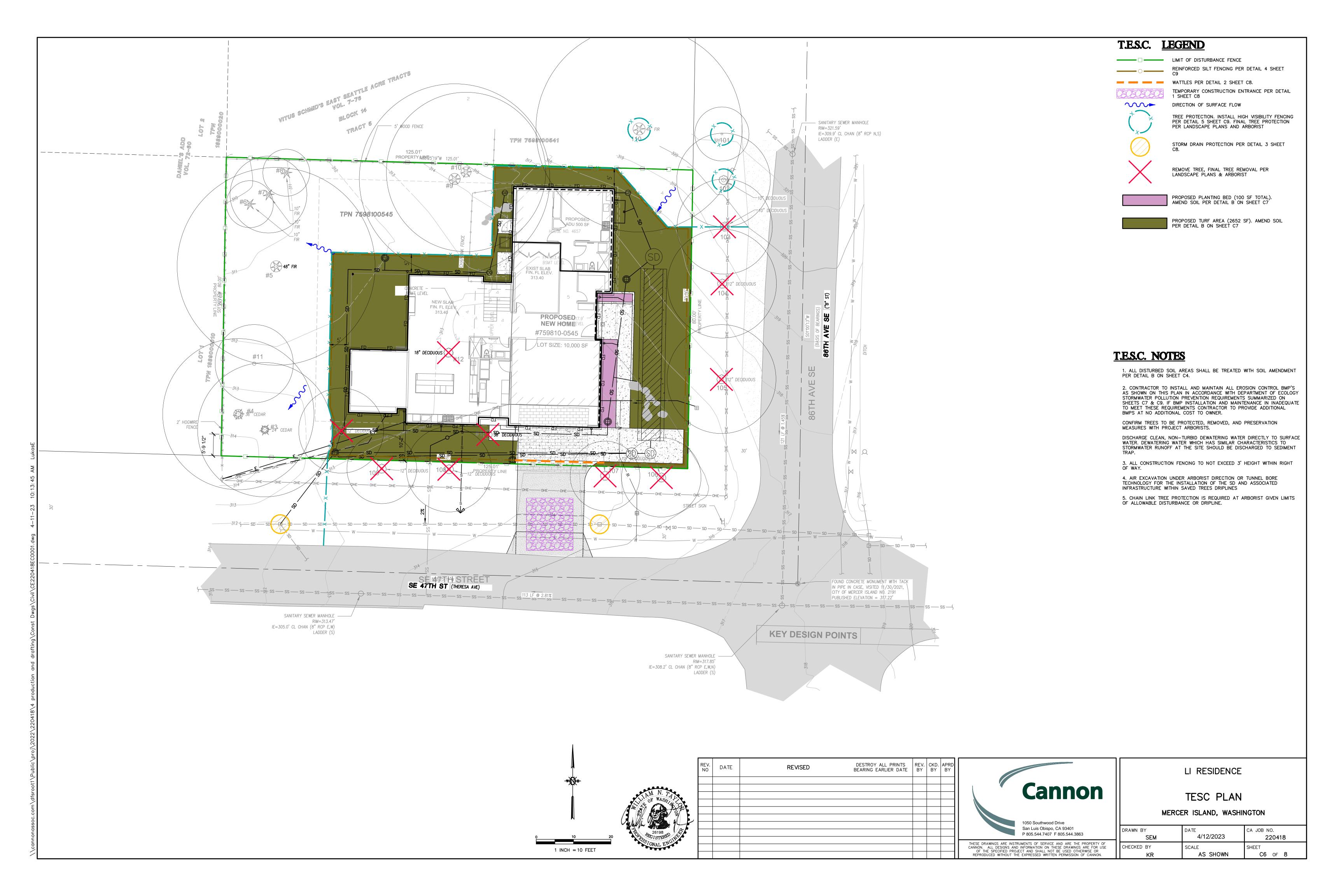
MYERS RESIDENTIAL CAST

MCIO33 OR APPROVED EQUAL

GRADING AND DRAINAGE DETAILS MERCER ISLAND, WASHINGTON

DRAWN BY CA JOB NO. 4/12/2023 220418 CHECKED BY C5 OF 8 AS SHOWN





3. THE BOUNDARIES OF THE CLEARING LIMITS SHOWN ON THIS PLAN SHALL BE CLEARLY FLAGGED IN THE FIELD PRIOR TO CONSTRUCTION. DURING THE CONSTRUCTION PERIOD, NO DISTURBANCE BEYOND THE FLAGGED CLEARING LIMITS SHALL BE PERMITTED. THE FLAGGING SHALL BE MAINTAINED BY THE APPLIC-ANT/CONTRACTOR FOR THE DURATION OF CONSTRUCTION.

4. THE ESC FACILITIES SHOWN ON THIS PLAN MUST BE CONSTRUCTED IN CONJUNCTION WITH ALL CLEAR-ING AND GRADING ACTIVITIES, AND IN SUCH A MANNER AS TO INSURE THAT SEDIMENT AND SEDIMENT-LADEN WATER DO NOT ENTER THE DRAINAGE SYSTEM, ROADWAYS, OR VIOLATE APPLICABLE WATER STANDARDS.

5. THE ESC FACILITIES SHOWN ON THIS PLAN ARE THE MINIMUM REQUIREMENTS FOR ANTICIPATED SITE CONDITIONS. DURING THE CONSTRUCTION PERIOD. THESE ESC FACILITIES SHALL BE UPGRADED AS NEEDED FOR UNEXPECTED STORM EVENTS AND TO ENSURE THAT SEDIMENT AND SEDIMENT-LADEN WATER DO NOT LEAVE THE SITE.

6. THE ESC FACILITIES SHALL BE INSPECTED DAILY BY THE APPLICANT/CONTRACTOR AND MAINTAINED AS NECESSARY TO ENSURE THEIR CONTINUED FUNCTIONING.

THE ESC FACILITIES ON INACTIVE SITES SHALL BE INSPECTED AND MAINTAINED A MINIMUM OF ONCE A MONTH OR WITHIN THE 48 HOURS FOLLOWING A MAJOR STORM EVENT.

8. AT NO TIME SHALL MORE THAN ONE FOOT OF SEDIMENT BE ALLOWED TO ACCUMULATE WITHIN A TRAPPED CATCH BASIN. ALL CATCH BASINS AND CONVEYANCE LINES SHALL BE CLEANED PRIOR TO PAV-ING. THE CLEANING OPERATION SHALL NOT FLUSH SEDIMENT LADEN WATER INTO THE DOWNSTREAM SYSTEM.

9. STABILIZED CONSTRUCTION ENTRANCES SHALL BE INSTALLED AT THE BEGINNING OF CONSTRUCTION AND MAINTAINED FOR THE DURATION OF THE PROJECT. ADDITIONAL MEASURES MAY BE REQUIRED TO INSURE THAT ALL PAVED AREAS ARE KEPT CLEAN FOR THE DURATION OF THE

BMP T5.13: Post-Construction Soil Quality and Depth

soil conditions and do not have an excessive percent of clay fines.

Establishing a minimum soil quality and depth is not the same as preservation of nat-

urally occurring soil and vegetation. However, establishing a minimum soil quality and

depth will provide improved on-site management of stormwater flow and water quality.

Soil organic matter can be attained through numerous materials such as compost, com-

materials used to meet the soil quality and depth BMP be appropriate and beneficial to

the plant cover to be established. Likewise, it is important that imported topsoils improve

• Soil retention. Retain, in an undisturbed state, the duff layer and native topsoil to

the maximum extent practicable. In any areas requiring grading remove and stock-

pile the duff layer and topsoil on site in a designated, controlled area, not adjacent

to public resources and critical areas, to be reapplied to other portions of the site

Soil quality. All areas subject to clearing and grading that have not been covered

tural fill or slope shall, at project completion, demonstrate the following:

the upper material to avoid stratified layers, where feasible.

2. Mulch planting beds with 2 inches of organic material

Turf: 2,652 SF x 5.4 CY \div 1,000 SF = $\frac{14}{1}$ CY

Total Quantity = 15 CY

Scarification depth: 8 inches

Planting beds: 100 SF x 9.3 CY \div 1,000 SF= 1 CY

Stockpile and amend

by impervious surface, incorporated into a drainage facility or engineered as struc-

1. A topsoil layer with a minimum organic matter content of 10% dry weight in

planting beds, and 5% organic matter content in turf areas, and a pH from 6.0

to 8.0 or matching the pH of the undisturbed soil. The topsoil layer shall have

incorporation of amendments needed to meet the criteria. Subsoils below the

topsoil layer should be scarified at least 4 inches with some incorporation of

a minimum depth of eight inches except where tree roots limit the depth of

posted woody material, biosolids, and forest product residuals. It is important that the

This BMP can be considered infeasible on till soil slopes greater than 33 percent.

Applications and Limitations

CONSTRUCTION SEQUENCE

HOLD THE PRE-CONSTRUCTION MEETING.

- 1. POST SIGN WITH NAME AND PHONE NUMBER OF ESC SUPERVISOR.
- 2. FENCE CLEARING LIMITS.
- 3. INSTALL CATCH BASIN PROTECTION.
- 4. GRADE AND INSTALL CONSTRUCTION ENTRANCES.
- 5. INSTALL PERIMETER PROTECTION (SILT FENCE, BRUSH BARRIER, ETC.). 6. CONSTRUCT SURFACE WATER CONTROLS SIMULTANEOUSLY WITH
- CLEARING AND GRADING FOR PROJECT DEVELOPMENT. 7. INSTALL STORM DRAINAGE AND SANITARY SEWER SYSTEMS.
- 8. INSTALL WATER SYSTEM, IRRIGATION, AND DRY UTILITIES AS SHOWN ON
- 9. INSTALL ASPHALT CONCRETE PAVEMENT, CURBING AND CEMENT CONCRETE AS SHOWN ON PLANS.
- 10. MAINTAIN EROSION CONTROL MEASURES IN ACCORDANCE WITH CITY OF MERCER ISLAND STANDARDS AND MANUFACTURER'S RECOMMENDATIONS.
- 11. RELOCATE EROSION CONTROL MEASURES, OR INSTALL NEW MEASURES SO THAT AS SITE CONDITIONS CHANGE, THE EROSION AND SEDIMENT CONTROL IS ALWAYS IN ACCORDANCE WITH THE CITY OF MERCER ISLAND CLEARING AND GRADING STANDARDS.
- 12. COVER ALL AREAS THAT WILL BE UNWORKED FOR MORE THAN SEVEN DAYS DURING THE WET SEASON WITH STRAW, WOOD FIBER MULCH, COMPOST, PLASTIC SHEETING, OR EQUIVALENT
- 13. STABILIZED ALL AREAS WITHIN SEVEN DAYS OF REACHING FINAL GRADE.
- 14. SEED, SOD, STABILIZE, OR COVER ANY AREAS TO REMAIN UNWORKED FOR MORE THAN 30 DAYS.
- 15. UPON COMPLETION OF THE PROJECT, STABILIZED ALL DISTURBED AREAS

3. Use compost and other materials that meet these organic content require-

a. The organic content for "pre-approved" amendment rates can be met only using compost meeting the compost specification for BMP T7.30: Bioretention Cells, Swales, and Planter Boxes (p.959), with the excep-

and a carbon to nitrogen ratio below 25:1.

The carbon to nitrogen ratio may be as high as 35:1 for plantings composed entirely of plants native to the Puget Sound Lowlands region.

b. Calculated amendment rates may be met through use of composted material meeting (a.) above; or other organic materials amended to meet the carbon to nitrogen ratio requirements, and not exceeding the contaminant limits identified in Table 220-B, Testing Parameters, in

The resulting soil should be conducive to the type of vegetation to be established.

- Implementation Options: The soil quality design guidelines listed above can be met by using one of the methods listed below:
- during construction.
- or at custom calculated rates based on tests of the soil and amendment.
- Stockpiled topsoil must also be amended if needed to meet the organic matter or depth requirements, either at a default "pre-approved" rate or at a custom calculated rate.

More than one method may be used on different portions of the same site. Soil that already meets the depth and organic matter quality standards, and is not com-

tion that the compost may have up to 35% biosolids or manure.

The compost must also have an organic matter content of 40% to 65%,

WAC 173-350-220.

- 1. Leave undisturbed native vegetation and soil, and protect from compaction
- 2. Amend existing site topsoil or subsoil either at default "pre-approved" rates,
- 3. Stockpile existing topsoil during grading, and replace it prior to planting.
- 4. Import topsoil mix of sufficient organic content and depth to meet the require-

pacted, does not need to be amended.

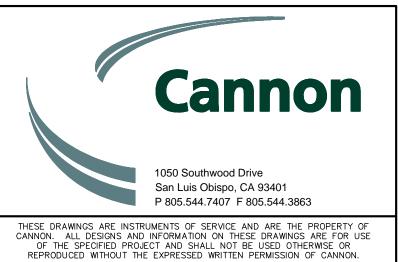
SOIL QUALITY AND DEPTH

BMPT7.3

Product:



V.	DATE	REVISED	DESTROY ALL PRINTS BEARING EARLIER DATE	REV. BY	CKD. BY	APRD BY	
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GUIDE TO MULCH MATERIALS, RATES AND USE

REMARKS

COST-EFFECTIVE PROTECTION WHEN APPLIED WITH

GENERALLY REQUIRES GREATER THICKNESS THAN

COVERING WITH NETTING. BLOWN STRAW ALWAYS

HAS TO BE HELD IN PLACE WITH A TACKIFIER AS

HOWEVER, HAS SEVERAL DEFICIENCIES THAT SHOULD

BE CONSIDERED WHEN SELECTING MULCH MATERIALS.

SIGNIFICANT LONG-TERM BENEFITS. STRAW SHOULD

BENEFITS ARE UNAVAILABLE LOCALLY. IT SHOULD

HIGH-WATER ELEVATION OF SURFACE WATERS (DUE

SHALL BE APPLIED WITH HYDROMULCHER. SHALL

THE APPLICATION RATE IS AT LEAST DOUBLED.

FIBERS LONGER THAN ABOUT ¾-1 INCH CLOG

NOT BE USED WITHOUT SEED AND TACKIFIER UNLESS

HYDROMULCH EQUIPMENT. FIBERS SHOULD BE KEPT

MORE EFFECTIVE CONTROL CAN BE OBTAINED BY

INCREASING THICKNESS TO 3". EXCELLENT MULCH

BECAUSE IT CAN BE DIRECTLY SEEDED OR TILLED

IS MORE STABLE AND PRACTICAL TO USE IN WET

AREAS AND DURING RAINY WEATHER CONDITIONS.

THIS IS A COST-EFFECTIVE WAY TO DISPOSE OF

DEBRIS FROM CLEARING AND GRUBBING, AND IT

BURNING. GENERALLY, IT SHOULD NOT BE USED ON

TENDENCY TO BE TRANSPORTED BY RUNOFF. IT IS

NOT RECOMMENDED WITHIN 200 FEET OF SURFACE

WATERS. IF SEEDING IS EXPECTED SHORTLY AFTER

VEGETATION MAY TIE UP NUTRIENTS IMPORTANT TO

THIS MATERIAL IS OFTEN CALLED "HOG OR HOGGED

STABILIZED CONSTRUCTION ENTRANCES (BMP C105)

AND AS A MULCH. THE USE OF MULCH ULTIMATELY

SOURCE AND COMPOSITION OF WOODBASED MULCHES.

ITS PREPARATION TYPICALLY DOES NOT PROVIDE ANY

INCLUSION OF WEED PLANTS OR SEEDS SHOULD BE

WEED SEED CONTROL, SO EVIDENCE OF RESIDUAL

ELIMINATES THE PROBLEMS ASSOCIATED WITH

SLOPES ABOVE APPROX. 10% BECAUSE OF ITS

MULCH, THE DECOMPOSITION OF THE CHIPPED

FUEL." IT IS USABLE AS A MATERIAL FOR

IMPROVES THE ORGANIC MATTER IN THE SOIL.

SPECIAL CAUTION IS ADVISED REGARDING THE

VEGETATION IN ITS COMPOSITION OR KNOWN

MONITORED AND PREVENTED (OR MINIMIZED).

INTO SOIL AS AN AMENDMENT. COMPOSTED MULCH

HAS A COARSER SIZE GRADATION THAN COMPOST. IT

FOR PROTECTING FINAL GRADES UNTIL LANDSCAPING

IT OFTEN INTRODUCES AND/OR ENCOURAGES THE

PROPAGATION OF WEED SPECIES AND IT HAS NO

BE USED ONLY IF MULCHES WITH LONG-TERM

ALSO NOT BE USED WITHIN THE ORDINARY

TO FLOTATION).

TO LESS THAN 3/4 INCH.

GRASS ESTABLISHMENT.

EVEN LIGHT WINDS WILL BLOW IT AWAY. STRAW,

BLOWN STRAW. THE THICKNESS OF STRAW MAY BE

REDUCED BY HALF WHEN USED IN CONJUNCTION WITH

SEEDING. IN WINDY AREAS STRAW MUST BE HELD IN

ADEQUATE THICKNESS. HAND-APPLICATION

PLACE BY CRIMPING, USING A TACKIFIER, OR

APPLICATION RATES

BALES PER 1000SF

OR 2-3 TONS PER

APPOX. 25-30 LBS

PER 1000 SF OR

1500-2000 LBS

2" THICK MIN.;

2" MINIMUM

2" THICK: APPROX.

100 TONS PER

ACRE (APPROX.

800 LBS. PER

CUBIC YARD)

THICKNESS

APPROX. 100 TONS

PER ACRE (APPROX.

800 LBS PER YARD)

PER ACRE

2"-3" THICK: 5

ACRE

QUALITY STANDARDS

AIR DRIED; FREE

SEED & COARSE

UNDESIRABLE

MATERIAL.

NO GROWTH

NO VISIBLE WATER

OR DUST DURING

HANDLING. MUST

BE PURCHASED

FROM SUPPLIER

HANDLING

EXEMPT).

SHALL BE

WITH SOLID WASTE

PERMIT (UNLESS

AVERAGE SIZE

FINES TO 6

FOR TEXTURE,

INTERLOCKING

PROPERTIES.

VARIATION, AND

SEVERAL INCHES.

GRADATIONS FROM

INCHES IN LENGTH

NO VISIBLE WATER

HANDLING. MUST

FROM A SUPPLIER

WASTE HANDLING

PERMIT OR ONE

EXEMPT FROM

SOLID WASTE

REGULATIONS.

OR DUST DURING

BE PURCHASED

WITH A SOLID

INHIBITING

FACTORS.

FROM

MATERIAL

HYDROMULCH

COMPOSTED

MULCH AND

CHIPPED SITE

WOOD-BASED

MULCH

VEGETATION

COMPOST

STRAW

LI RESIDENCE

TESC NOTES AND STANDARD DETAILS

MERCER ISLAND, WASHINGTON

DRAWN BY SEM	DATE 4/12/2023	CA JOB NO. 220418
CHECKED BY	SCALE	SHEET
KB	AS SHOWN	C7 OF 8

Purpose

Stabilized Construction entrances are established to reduce the amount of sediment transported onto paved roads by vehicles or equipment. This is done by constructing a stabilized pad of guarry spalls at entrances and exits for construction sites.

BMP C105: Stabilized Construction Entrance / Exit

Conditions of Use

Construction entrances shall be stabilized wherever traffic will be entering or leaving a construction site if paved roads or other paved areas are within 1,000 feet of the site.

For residential construction provide stabilized construction entrances for each residence, rather than only at the main subdivision entrance. Stabilized surfaces shall be of sufficient length/width to provide vehicle access/parking, based on lot size/configuration.

On large commercial, highway, and road projects, the designer should include enough extra materials in the contract to allow for additional stabilized entrances not shown in the initial Construction SWPPP. It is difficult to determine exactly where access to these projects will take place; additional materials will enable the contractor to install them where needed.

Design and Installation Specifications

See Figure II-4.1.1 Stabilized Construction Entrance (p.273) for details. Note: the 100' minimum length of the entrance shall be reduced to the maximum practicable size when the size or configuration of the site does not allow the full length (100').

Construct stabilized construction entrances with a 12-inch thick pad of 4-inch to 8-inch quarry spalls, a 4-inch course of asphalt treated base (ATB), or use existing pavement. Do not use crushed concrete, cement, or calcium chloride for construction entrance stabilization because these products raise pH levels in stormwater and concrete discharge to surface waters of the State is prohibited.

A separation geotextile shall be placed under the spalls to prevent fine sediment from pumping up into the rock pad. The geotextile shall meet the following standards:

Grab Tensile Strength (ASTM D4751)	200 psi min.
Grab Tensile Elongation (ASTM D4632)	30% max.
Mullen Burst Strength (ASTM D3786-80a)	400 psi min.
AOS (ASTM D4751)	20-45 (U.S. standard sieve size)

- Consider early installation of the first lift of asphalt in areas that will paved; this can be used as a stabilized entrance. Also consider the installation of excess concrete as a stabilized entrance. During large concrete pours, excess concrete is often available for this purpose.
- Fencing (see BMP C103: High Visibility Fence (p.269)) shall be installed as necessary to restrict traffic to the construction entrance.
- Whenever possible, the entrance shall be constructed on a firm, compacted subgrade. This can substantially increase the effectiveness of the pad and reduce the need for maintenance.
- Construction entrances should avoid crossing existing sidewalks and back of walk drains if at all possible. If a construction entrance must cross a sidewalk or back of walk drain, the full length of the sidewalk and back of walk drain must be covered and protected from sediment leaving the site.

Maintenance Standards

Quarry spalls shall be added if the pad is no longer in accordance with the spe-

- If the entrance is not preventing sediment from being tracked onto pavement, then alternative measures to keep the streets free of sediment shall be used. This may include replacement/cleaning of the existing quarry spalls, street sweeping, an increase in the dimensions of the entrance, or the installation of a wheel wash.
- Any sediment that is tracked onto pavement shall be removed by shoveling or street sweeping. The sediment collected by sweeping shall be removed or stabilized on site. The pavement shall not be cleaned by washing down the street, except when high efficiency sweeping is ineffective and there is a threat to public safety. If it is necessary to wash the streets, the construction of a small sump to contain the wash water shall be considered. The sediment would then be washed into the sump where it can be controlled.
- Perform street sweeping by hand or with a high efficiency sweeper. Do not use a non-high efficiency mechanical sweeper because this creates dust and throws soils into storm systems or conveyance ditches.
- Any quarry spalls that are loosened from the pad, which end up on the roadway shall be removed immediately.
- If vehicles are entering or exiting the site at points other than the construction entrance(s), fencing (see BMP C103) shall be installed to control traffic.
- Upon project completion and site stabilization, all construction accesses intended as permanent access for maintenance shall be permanently stabilized.

BMP C235: Wattles

Purpose

Wattles are temporary erosion and sediment control barriers consisting of straw, compost, or other material that is wrapped in netting made of natural plant fiber or similar encasing material. They reduce the velocity and can spread the flow of rill and sheet runoff, and can capture and retain sediment.

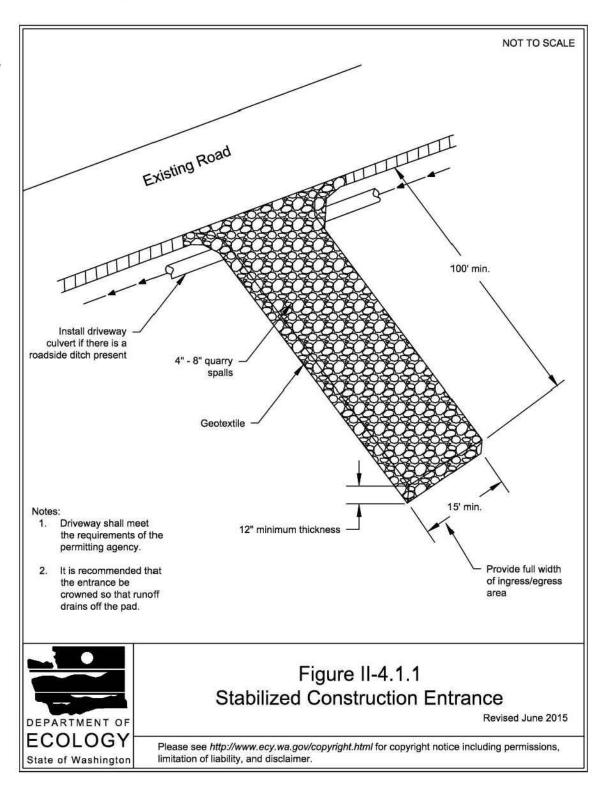
Conditions of Use

- Wattles shall consist of cylinders of plant material such as weed-free straw, coir, wood chips, excelsior, or wood fiber or shavings encased within netting made of natural plant fibers unaltered by synthetic materials.
- · Use wattles:
- In disturbed areas that require immediate erosion protection.
- On exposed soils during the period of short construction delays, or over winter months.
- o On slopes requiring stabilization until permanent vegetation can be established.
- The material used dictates the effectiveness period of the wattle. Generally, wattles are effective for one to two seasons.
- Prevent rilling beneath wattles by entrenching and overlapping wattles to prevent water from passing between them.

Design Criteria

- See Figure II-3.24: Wattles for typical construction details.
- Wattles are typically 8 to 10 inches in diameter and 25 to 30 feet in length.
- Install wattles perpendicular to the flow direction and parallel to the slope contour.

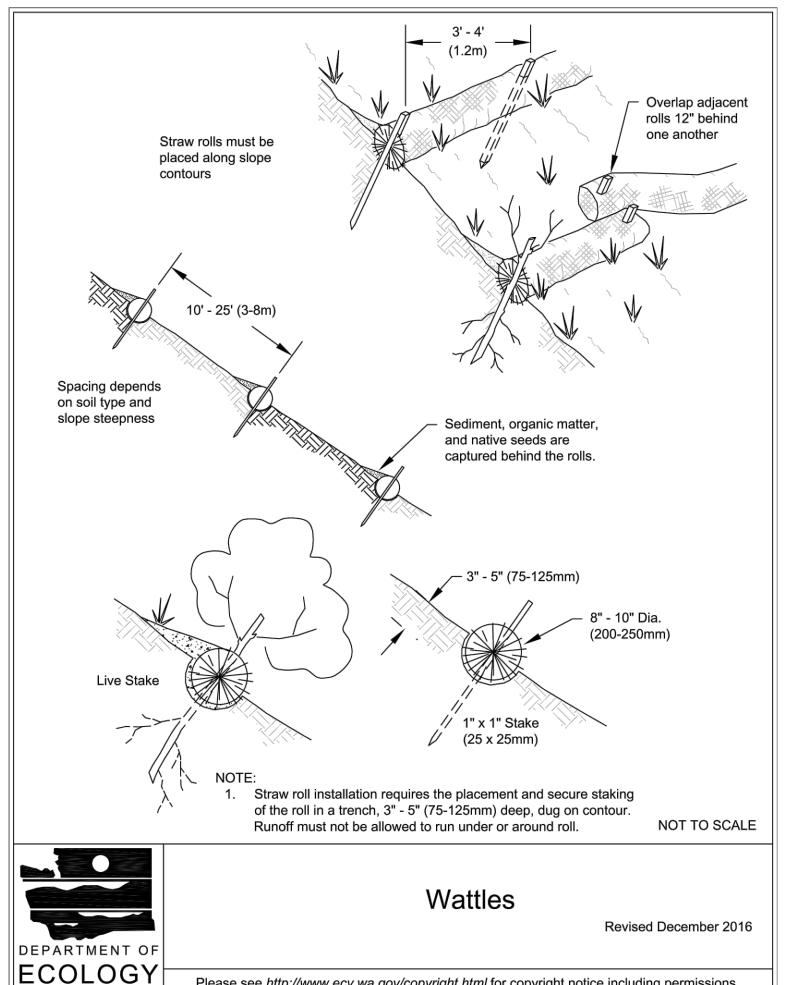
Figure II-4.1.1 Stabilized Construction Entrance



STABILIZED CONSTRUCTION ENTRANCE

 Place wattles in shallow trenches, staked along the contour of disturbed or newly constructed slopes. Dig narrow trenches across the slope (on contour) to a depth of 3- to 5-inches on clay soils and soils with gradual slopes. On loose soils, steep slopes, and areas with high rainfall, the trenches should be dug to a depth of 5- to 7- inches, or 1/2 to 2/3 of the thickness of the wattle.

 Start building trenches and installing wattles from the base of the slope and work up. Spread excavated material evenly along the uphill slope and compact it using hand tamping or other methods.



BMP C235: Wattles

- Construct trenches at intervals of 10- to 25-feet depending on the steepness of the slope, soil type, and rainfall. The steeper the slope the closer together the trenches.
- Install the wattles snugly into the trenches and overlap the ends of adjacent wattles 12 inches behind one another.

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Install stakes at each end of the wattle, and at 4-foot centers along entire length of wattle.

limitation of liability, and disclaimer.

- If required, install pilot holes for the stakes using a straight bar to drive holes through the wattle and into the
- Wooden stakes should be approximately 0.75 x 0.75 x 24 inches min. Willow cuttings or 3/8-inch rebar can also be used for stakes.
- Stakes should be driven through the middle of the wattle, leaving 2 to 3 inches of the stake protruding above the wattle.

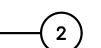
Maintenance Standards

WATTLES

DATE

State of Washington

- Wattles may require maintenance to ensure they are in contact with soil and thoroughly entrenched, especially after significant rainfall on steep sandy soils.
- Inspect the slope after significant storms and repair any areas where wattles are not tightly abutted or water has scoured beneath the wattles.



BMP C220: Storm Drain Inlet Protection

Purpose

Storm drain inlet protection prevents coarse sediment from entering drainage systems prior to permanent stabilization of the disturbed area.

Conditions of Use

Use storm drain inlet protection at inlets that are operational before permanent stabilization of the disturbed drainage area. Provide protection for all storm drain inlets downslope and within 500 feet of a disturbed or construction area, unless conveying runoff entering catch basins to a sediment pond or trap.

Also consider inlet protection for lawn and yard drains on new home construction. These small and numerous drains coupled with lack of gutters in new home construction can add significant amounts of sediment into the roof drain system. If possible delay installing lawn and yard drains until just before landscaping or cap these drains to prevent sediment from entering the system until completion of landscaping. Provide 18inches of sod around each finished lawn and yard drain.

Table II-4.2.2 Storm Drain Inlet Protection (p.358) lists several options for inlet protection. All of the methods for storm drain inlet protection tend to plug and require a high frequency of maintenance. Limit drainage areas to one acre or less. Possibly provide emergency overflows with additional end-of-pipe treatment where stormwater ponding would cause a hazard.

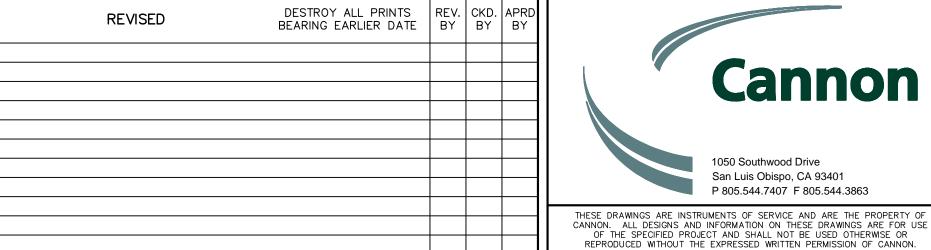
Type of Inlet Protection	Emergency Overflow	Applicable for Paved/ Earthen Surfaces	Conditions of Use
Drop Inlet Prote	ction		\$
Excavated drop	Yes, tem- porary flood- ing will occur	Earthen	Applicable for heavy flows. Easy to maintain. Large area Requirement: 30'x30'/acre
Block and gravel drop inlet protection	Yes	Paved or Earthen	Applicable for heavy concentrated flows. Will not pond.
Gravel and wire drop inlet pro- tection	No		Applicable for heavy concentrated flows. Will pond. Can withstand traffic.
Catch basin fil- ters	Yes	Paved or Earthen	Frequent Maintenance required.
Curb Inlet Prote	ction		
Curb inlet pro- tection with wooden weir	Small capacity overflow	Paved	Used for sturdy, more compact installation.
Block and gravel curb inlet protection	Yes	Paved	Sturdy, but limited filtration.
Culvert Inlet Pro	tection		
Culvert inlet Sed- iment trap			18 month expected life.

Maintenance Standards

- Inspect catch basin filters frequently, especially after storm events. Clean and replace clogged inserts. For systems with clogged stone filters: pull away the stones from the inlet and clean or replace. An alternative approach would be to use the clogged stone as fill and put fresh stone around the inlet.
- Do not wash sediment into storm drains while cleaning. Spread all excavated material evenly over the surrounding land area or stockpile and stabilize as appro-

STORM DRAIN INLET PROTECTTION

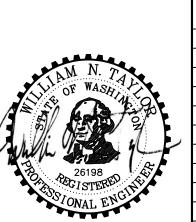




LI RESIDENCE

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Cannon

Purpose

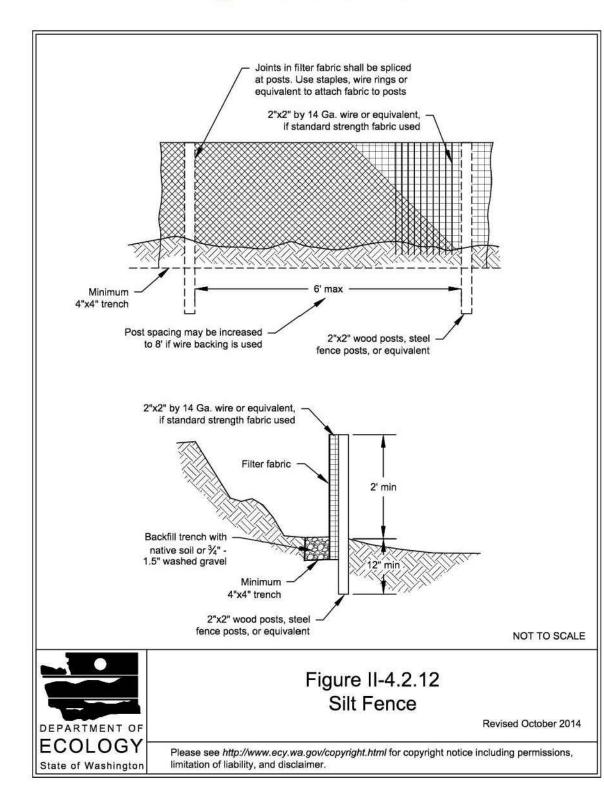
Use of a silt fence reduces the transport of coarse sediment from a construction site by providing a temporary physical barrier to sediment and reducing the runoff velocities of overland flow. See Figure II-4.2.12 Silt Fence (p.369) for details on silt fence construction.

Conditions of Use

Silt fence may be used downslope of all disturbed areas.

- Silt fence shall prevent soil carried by runoff water from going beneath, through, or over the top of the silt fence, but shall allow the water to pass through the fence.
- Silt fence is not intended to treat concentrated flows, nor is it intended to treat substantial amounts of overland flow. Convey any concentrated flows through the drainage system to a sediment pond.
- Do not construct silt fences in streams or use in V-shaped ditches. Silt fences do not provide an adequate method of silt control for anything deeper than sheet or overland flow.

Figure II-4.2.12 Silt Fence



Design and Installation Specifications

- Use in combination with sediment basins or other BMPs.
- Maximum slope steepness (normal (perpendicular) to fence line) 1H:1V.
- Maximum sheet or overland flow path length to the fence of 100 feet.
- Do not allow flows greater than 0.5 cfs.
- The geotextile used shall meet the following standards. All geotextile properties listed below are minimum average roll values (i.e., the test result for any sampled roll in a lot shall meet or exceed the values shown in Table II-4.2.3 Geotextile Standards (p.370):

Table II-4.2.3 Geotextile Standards

Polymeric Mesh AOS	0.60 mm maximum for slit film woven (#30 sieve).
	0.30 mm maximum for all other geotextile types (#50 sieve).
(ASTM D4751)	0.15 mm minimum for all fabric types (#100 sieve).
Water Permittivity	0.02 sec ⁻¹ minimum
(ASTM D4491)	0.02 sec minimum
Grab Tensile Strength	180 lbs. Minimum for extra strength fabric.
(ASTM D4632)	100 lbs minimum for standard strength fabric.
Grab Tensile Strength	200/ maximum
(ASTM D4632)	30% maximum
Ultraviolet Resistance	700/
(ASTM D4355)	70% minimum

- Support standard strength fabrics with wire mesh, chicken wire, 2-inch x 2-inch wire, safety fence, or jute mesh to increase the strength of the fabric. Silt fence materials are available that have synthetic mesh backing attached.
- Filter fabric material shall contain ultraviolet ray inhibitors and stabilizers to provide a minimum of six months of expected usable construction life at a temperature range of 0°F. to 120°F.
- One-hundred percent biodegradable silt fence is available that is strong, long lasting, and can be left in place after the project is completed, if permitted by local regulations.
- Refer to Figure II-4.2.12 Silt Fence (p.369) for standard silt fence details. Include the following standard Notes for silt fence on construction plans and specifications:
 - 1. The contractor shall install and maintain temporary silt fences at the locations shown in the Plans.
 - 2. Construct silt fences in areas of clearing, grading, or drainage prior to starting those activities.
- 3. The silt fence shall have a 2-feet min. and a 21/2-feet max. height above the original ground surface.
- 4. The filter fabric shall be sewn together at the point of manufacture to form filter fabric lengths as required. Locate all sewn seams at support posts. Alternatively, two sections of silt fence can be overlapped, provided the Contractor can demonstrate, to the satisfaction of the Engineer, that the overlap is long enough and that the adjacent fence sections are close enough together to prevent silt laden water from escaping through the fence at the overlap.
- 5. Attach the filter fabric on the up-slope side of the posts and secure with staples, wire, or in accordance with the manufacturer's recommendations Attach the filter fabric to the posts in a manner that reduces the potential for
- 6. Support the filter fabric with wire or plastic mesh, dependent on the properties of the geotextile selected for use. If wire or plastic mesh is used, fasten the mesh securely to the up-slope side of the posts with the filter fabric up-slope of the mesh.
- 7. Mesh support, if used, shall consist of steel wire with a maximum mesh spacing of 2-inches, or a prefabricated polymeric mesh. The strength of the wire or polymeric mesh shall be equivalent to or greater than 180 lbs. grab tensile strength. The polymeric mesh must be as resistant to the same level of ultraviolet radiation as the filter fabric it supports.
- 8. Bury the bottom of the filter fabric 4-inches min. below the ground surface. Backfill and tamp soil in place over the buried portion of the filter fabric, so that no flow can pass beneath the fence and scouring cannot occur. When wire or polymeric back-up support mesh is used, the wire or polymeric mesh shall extend into the ground 3-inches min.
- 9. Drive or place the fence posts into the ground 18-inches min. A 12-inch min. depth is allowed if topsoil or other soft subgrade soil is not present and 18inches cannot be reached. Increase fence post min. depths by 6 inches if the fence is located on slopes of 3H:1V or steeper and the slope is perpendicular to the fence. If required post depths cannot be obtained, the posts shall be adequately secured by bracing or guying to prevent overturning of the fence due to sediment loading.
- 10. Use wood, steel or equivalent posts. The spacing of the support posts shall

REINFORCED SILT FENCE

be a maximum of 6-feet. Posts shall consist of either:

- Wood with dimensions of 2-inches by 2-inches wide min. and a 3-feet min. length. Wood posts shall be free of defects such as knots, splits, or
- No. 6 steel rebar or larger.
- ASTM A 120 steel pipe with a minimum diameter of 1-inch.
- U, T, L, or C shape steel posts with a minimum weight of 1.35 lbs./ft.
- Other steel posts having equivalent strength and bending resistance to the post sizes listed above.
- 11. Locate silt fences on contour as much as possible, except at the ends of the fence, where the fence shall be turned uphill such that the silt fence captures the runoff water and prevents water from flowing around the end of the fence
- 12. If the fence must cross contours, with the exception of the ends of the fence, place gravel check dams perpendicular to the back of the fence to minimize concentrated flow and erosion. The slope of the fence line where contours must be crossed shall not be steeper than 3H:1V.
 - Gravel check dams shall be approximately 1-foot deep at the back of the fence. Gravel check dams shall be continued perpendicular to the fence at the same elevation until the top of the check dam intercepts the ground surface behind the fence.
 - Gravel check dams shall consist of crushed surfacing base course, gravel backfill for walls, or shoulder ballast. Gravel check dams shall be located every 10 feet along the fence where the fence must cross con-

Maintenance Standards

- Repair any damage immediately.
- Intercept and convey all evident concentrated flows uphill of the silt fence to a sediment pond.
- Check the uphill side of the fence for signs of the fence clogging and acting as a barrier to flow and then causing channelization of flows parallel to the fence. If this occurs, replace the fence or remove the trapped sediment.
- Remove sediment deposits when the deposit reaches approximately one-third the height of the silt fence, or install a second silt fence.
- Replace filter fabric that has deteriorated due to ultraviolet breakdown.

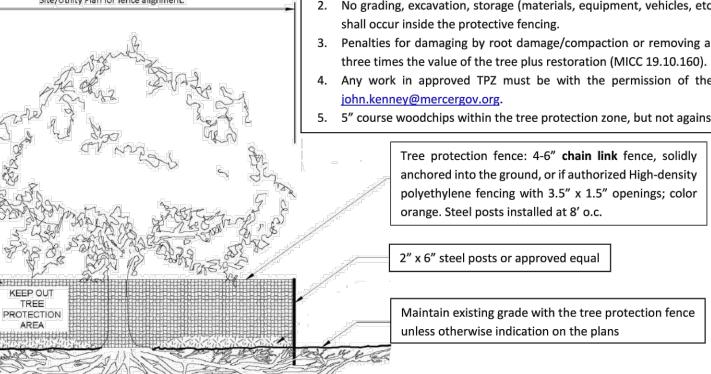
TREE PROTECTION AREA (TPZ) **KEEP OUT!**

DO NOT REMOVE OR ADJUST THE APPROVED LOCATION OF THIS TREE PROTECTION AREA

Trees enclosed by this fence are protected and are subject to the conditions of the tree permit. Violation of tree conditions may lead to:

- RE Inspection Fees/financial penalties 3. Arborist reports recommending mitigation 1. No pruning shall be performed unless under the direction of the Project Arborist. Including limbing Crown drip line or other limit of Tree Protection area. See Site/Utility Plan for fence alignment. shall occur inside the protective fencing.

 - No grading, excavation, storage (materials, equipment, vehicles, etc.), or other unpermitted activity
 - 3. Penalties for damaging by root damage/compaction or removing a saved tree may be a fine up to three times the value of the tree plus restoration (MICC 19.10.160).
 - Any work in approved TPZ must be with the permission of the City Arborist (206) 275-7713,
 - 5. 5" course woodchips within the tree protection zone, but not against the tree trunk.



Any Work in the protected area must be with the permission of the City Arborist john.kenney@mercergov.org

BMP C103: High Visibility Fence

Purpose

Fencing is intended to:

1. Correction Notices or Stop Work Orders until compliance is achieved

- Restrict clearing to approved limits.
- 2. Prevent disturbance of sensitive areas, their buffers, and other areas required to be left undisturbed.
- 3. Limit construction traffic to designated construction entrances, exits, or internal
- 4. Protect areas where marking with survey tape may not provide adequate protection.

Conditions of Use

To establish clearing limits plastic, fabric, or metal fence may be used:

- At the boundary of sensitive areas, their buffers, and other areas required to be left
- As necessary to control vehicle access to and on the site.

Design and Installation Specifications

High visibility plastic fence shall be composed of a high-density polyethylene material and shall be at least four feet in height. Posts for the fencing shall be steel or wood and placed every 6 feet on center (maximum) or as needed to ensure rigidity. The fencing shall be fastened to the post every six inches with a polyethylene tie. On long continuous lengths of fencing, a tension wire or rope shall be used as a top stringer to prevent sagging between posts. The fence color shall be high visibility orange. The fence tensile strength shall be 360 lbs./ft. using the ASTM D4595 testing method.

If appropriate install fabric silt fence in accordance with BMP C233: Silt Fence (p.367) to act as high visibility fence. Silt fence shall be at least 3 feet high and must be highly visible to meet the requirements of this BMP.

Metal fences shall be designed and installed according to the manufacturer's spe-

Metal fences shall be at least 3 feet high and must be highly visible.

Fences shall not be wired or stapled to trees.

Maintenance Standards

If the fence has been damaged or visibility reduced, it shall be repaired or replaced immediately and visibility restored.

HIGH VISIBILITY FENCE

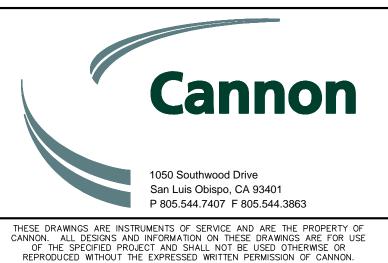


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C9 OF 8

PROPOSED REPLACEMENT TREES







A GOLDEN FULL MOON B RED LACE LEAF JAPANESE MAPLE

C JAPANESE MAPLE

TREE REPLANTING PER MICC19.10

- TREES CAN BE REPLANTED BENEATH THE CANOPY OF EXISTING TREES. -NEW TREES TO BE PLANTED NO LESS THAN 10 FEET FROM OTHER TREES, FENCES, STRUCTURES OR UTILITIES -50% OF NEW REPLACEMENT TREES TO BE NATIVE SPECIES -CONIFEROUS REPLACEMENT TREES MUST BE MIN. 6 FEET TALL -DECIDUOUS REPLACEMENT TREES MUST BE MIN. 1.5" DIA -REMOVING TREES LESS THAN 10" REQUIRES (1) REPLACEMENTS - REMOVING TREES 10 " UP TO 24" REQUIRES (2) REPLACEMENTS

TREE WATERING

AN IRRIGATION DRIP LINE / RING SYSTEM WILL BE PROVIDED FOR WATERING NEWLY PLANTED REPLACEMENT TREES.

ARBORIST REPORT

SEE ATTACHED ARBORIST REPORT FOR ADDITIONAL TREE PROTECTION AND ADDITIONAL INFORMATION

PROPOSED TREE TO BE REMOVED / REQ'D REPLACEMENT						
NO.	SPECIES	SIZE	REASON FOR REMOVAL	REQ'D REPLANTING		
#	CHERRY	17"	NEW CONSTRUCTION ACTIVITY	2		
#2	APPLE	9"	NEW CONSTRUCTION ACTIVITY	I		
#12	CHERRY	14"	NEW CONSTRUCTION ACTIVITY	2		
#103	PLUM	18"	NEW CONSTRUCTION ACTIVITY	I		
#104	PLUM	14"	NEW CONSTRUCTION ACTIVITY	I		
#105	PLUM	14"	NEW CONSTRUCTION ACTIVITY	I		
#106	PLUM	15"	NEW CONSTRUCTION ACTIVITY	I		
#107	PLUM	8"	NEW CONSTRUCTION ACTIVITY	0		
#108	APPLE	10"	NEW CONSTRUCTION ACTIVITY	I		
#109	APPLE	8"	NEW CONSTRUCTION ACTIVITY	I		
TOTAL REQUIRED TREE REPLACEMENT			13			

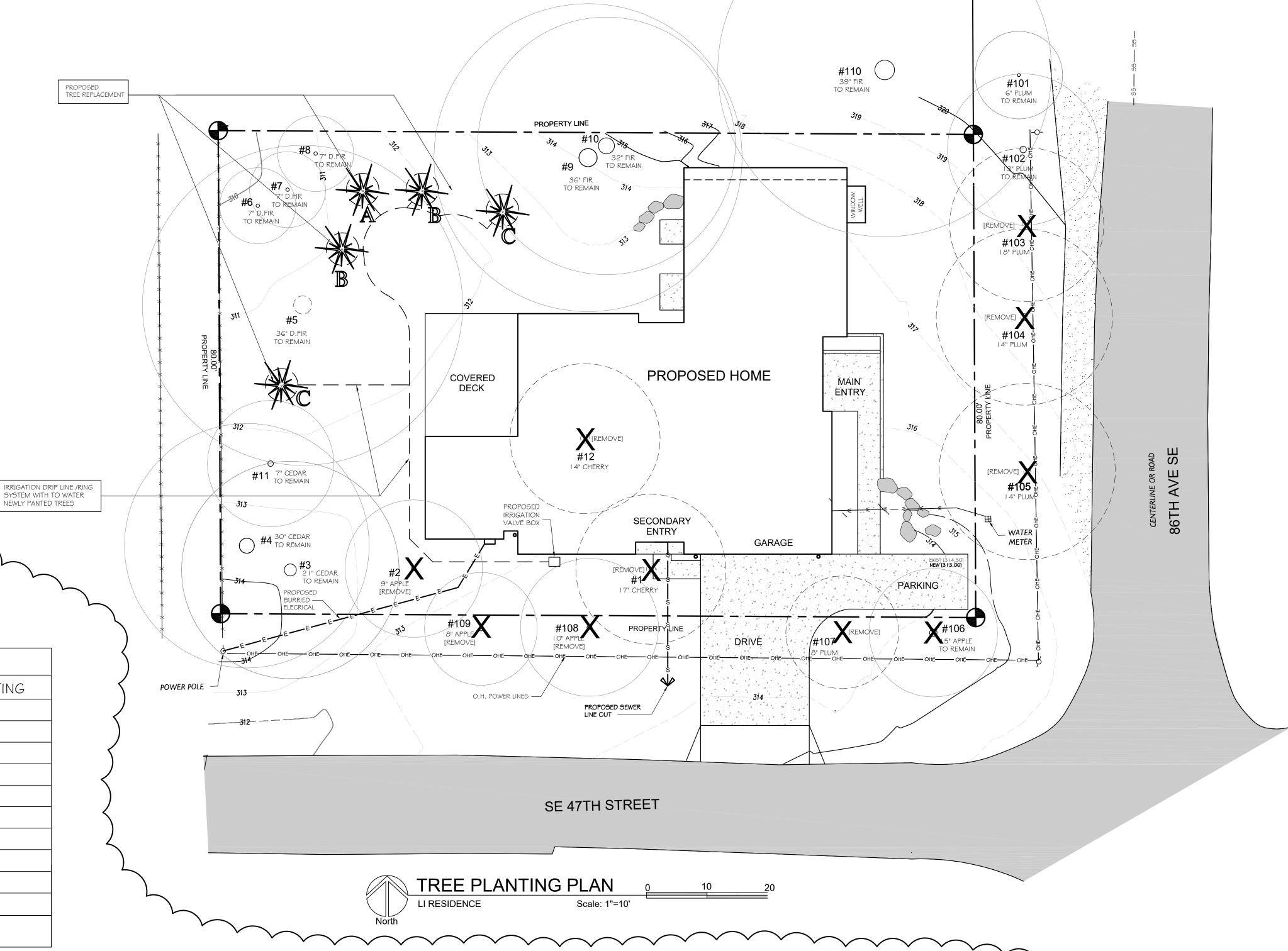
PROPOSED TREE TO BE PLANTED

-(5) TREES ARE PROPOSED TO BE REPLANTED WITHIN THE PROPERTY UNDER THE EXISTING TREES CANOPY 10' FROM EXISTING TREES, STRUCTURES AND FENCES. THIS STILL ALLOWS FOR A COMFORTABLE REAR YARDS SPACE FOR LAWN AND LANDSCAPE.

FEE IN LIEU OF REPLACEMENT

-(7) TREES ARE SELECTED FOR "FEE IN LIEU OF"

EXISTING UTILITIES LOCATED IN THE ROW LIMIT THE AREA OF REPLANTING AND THE HOME OWNER WISHED NOT TO HAVE TREES LOCATED UNDER POWER LINES THAT WILL EVENTUALLY REQUIRE UNSIGHTLY PRUNING AND TRIMMING.



PROPOSED TREE TO BE PLANTED					
PLANT ID	QUANTITY	REPLANTING TREE LIST	SIZE	TYPE	SUNLIGHT EXPOSURE
A	I	GOLDEN FULL MOON-(AUREUM)	1.5" DIA.	DECIDUOUS	LIGHT TO OPEN SHADE
\mathbb{B}	2	RED LACE LEAF JAPANESE MAPLE- (ORANGEOLA)	1.5" DIA.	DECIDUOUS	LIGHT O OPEN SHADE
\mathbb{C}	2	JAPANESE MAPLE- (SHINDESHOJO)	1.5" DIA.	DECIDUOUS	LIGHT TO DAPPLE SHADE
TOTAL	5		1		